Surface-groundwater flow numerical model for coastal barrier beach

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Motivation and Goals

• To develop a surface-groundwater flow model to better understand the effects of seepage on swash hydro-morphodynamics.

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- Validation of the model is by comparing the numerical results against the BARDEX II prototype-scale experiment.

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Variables involved

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Laplace's equation

$$\nabla \cdot \phi = 0, \qquad (3)$$

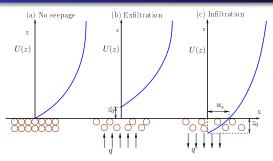
$$\phi = h + z_b,$$

$$\nabla = \frac{\partial^2}{\partial x^2} + \frac{\partial^2}{\partial z^2}.$$

$$q = -k \frac{\partial \phi}{\partial n} \quad (4)$$

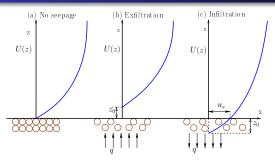
Variables involved





BBL model based on Cheng and Chiew (1998)

$$U(z) = \frac{u_*}{\kappa} \ln\left(\frac{z}{z_0}\right) + \frac{q}{4} \left[\frac{1}{\kappa} \ln\left(\frac{z}{z_0}\right)\right]^2$$
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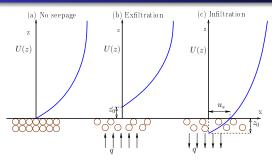


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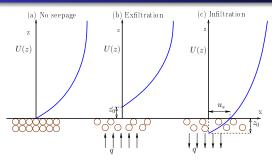


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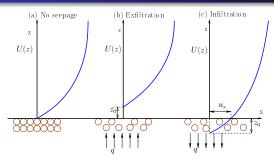


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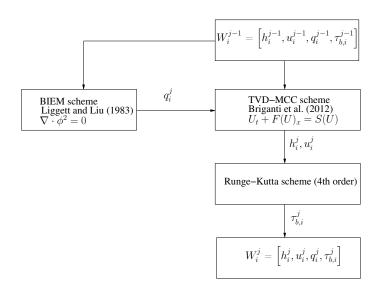
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$$\tau = \rho_w u_*^2 \tag{6}$$

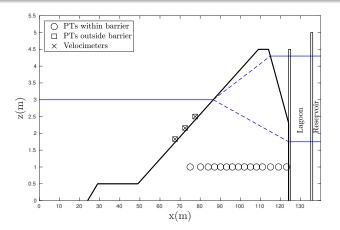
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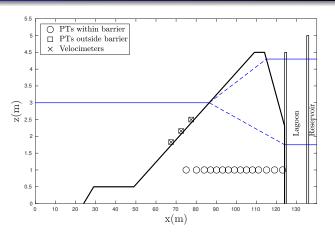
Coupled surface-groundwater flow model



The BARDEX II experimental set-up (Delta flume, Deltares, Netherlands)

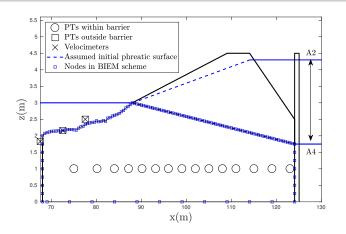


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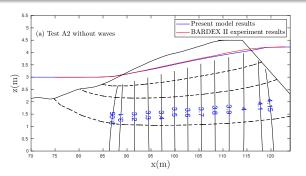
- Wavemaker at x = 0 m, $H_s = 0.8$ m and $T_p = 8$ s
- 2 test cases: A2 (sea level < lagoon level), A4 (sea level > lagoon level)

Numerical model set-up

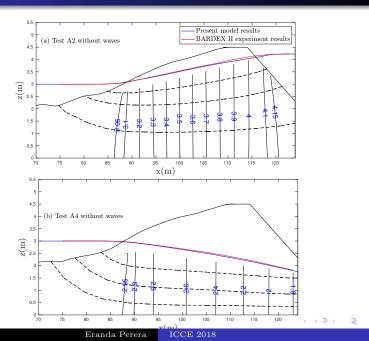


- Surface flow model: $\Delta x = 0.01$ m, $L_x = 56.5$ m, CN = 0.5, $h_{min} = 1 \times 10^{-3}$ m , $d_{50} = 0.43$ mm, $p_b = 0.4$, $k = 8 \times 10^{-4}$ ms⁻¹
- Groundwater flow model: $\Delta l \cos \alpha = 50 \Delta x$ m where $\tan \alpha = \frac{dz_b}{dx}$ (sea-side), $\Delta l = 0.5, 0.25, 0.5$ m (other sides)

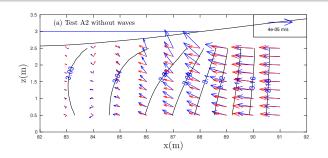
Phreatic surface validation - without waves

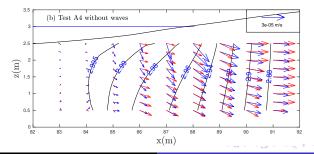


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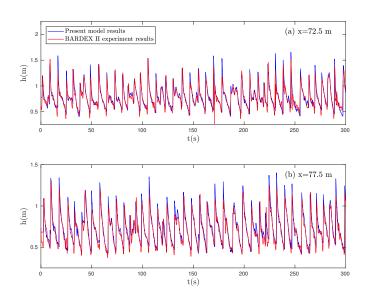


Pore velocities below the beach - without waves

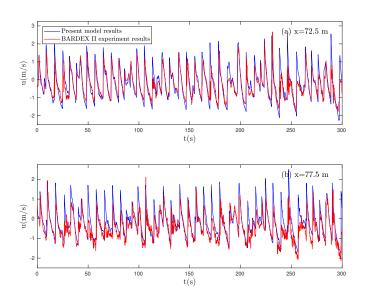




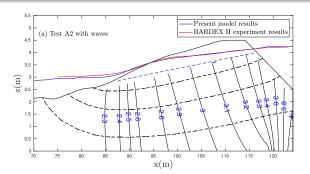
Surface flow validation - with waves



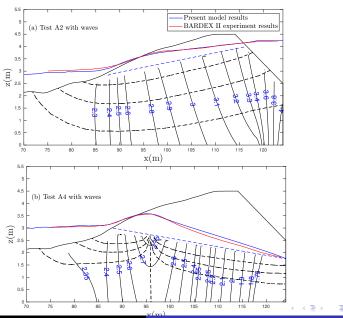
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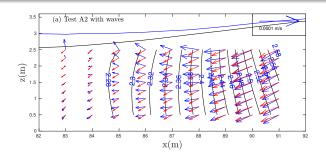
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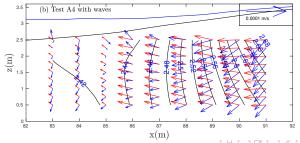


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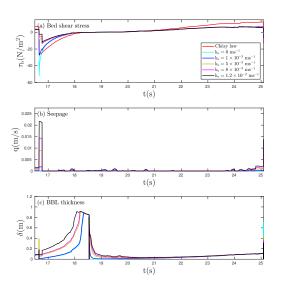


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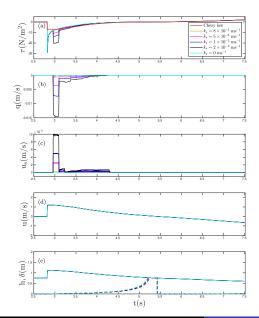


Exfiltration on BBL



- Increase of exfiltration rate:
 - increases BBL thickness,
 - 2 decreases bed shear stress.

Infiltration on BBL



- Increase of infiltration rate:
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Conclusion

• Model is capable of simulating barrier beach groundwater dynamics with reasonable accuracy.

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- Model is capable of simulating barrier beach groundwater dynamics with reasonable accuracy.
- Exfiltration reduces bed shear stress due to thickening of boundary layer.
- Infiltration increases bed shear stress due to thinning of boundary layer.

References



(1) Briganti, R., Dodd, N., Kelly, D., and Pokrajac, D. (2012)

An efficient and flexible solver for the simulation of the morphodynamics of fast evolving flows on coarse sediment beaches.

International Journal for Numerical Methods in Fluids 69(4), 859–877.



(2) Cheng, N.-S. and Chiew, Y.-M. (1998)

Modified logarithmic law for velocity distribution subjected to upward seepage.

Journal of Hydraulic Engineering 124(12), 1235 – 1241.



(3) Chen, X. and Chiew, Y.-M. (2004)

Velocity distribution of turbulent open-channel flow with bed suction.

Journal of Hydraulic Engineering 130(2), 140–148.



(4) Liggett, J. and Liu, P. (1983)

The boundary integral equation method for porous media flow.

Applied Ocean Research 122, 27-43.

Thank you for your attention!

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