

36TH INTERNATIONAL CONFERENCE ON COASTAL ENGINEERING 2018

Baltimore, Maryland | July 30 - August 3, 2018

The State of the Art and Science of Coastal Engineering

A new and fully automatic procedure for the identification and coupling of the overtopping events



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Motivations and background

- Prediction of individual wave overtopping volumes
- Determination of the overtopping flow characteristics across the structure crest
- Assessment of the maximum loads
- Average overtopping discharge



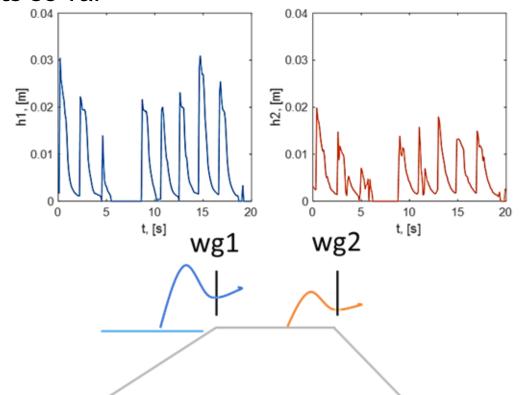


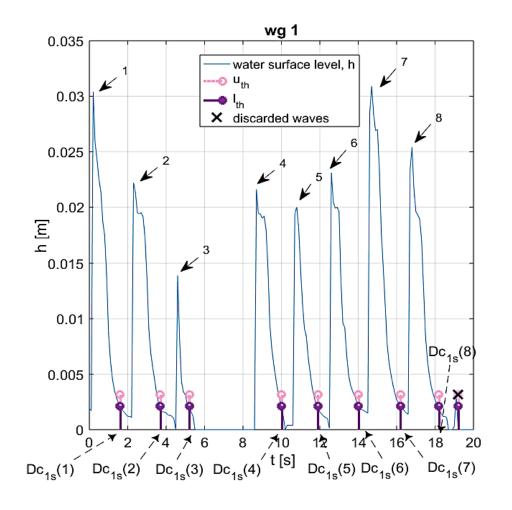




Motivations and background

- Unaffordable inaccuracy of the existing automatic procedures (Hughes & Thornton, 2016)
- No procedure for the coupling of the overtopping events at consecutive gauges exists so far









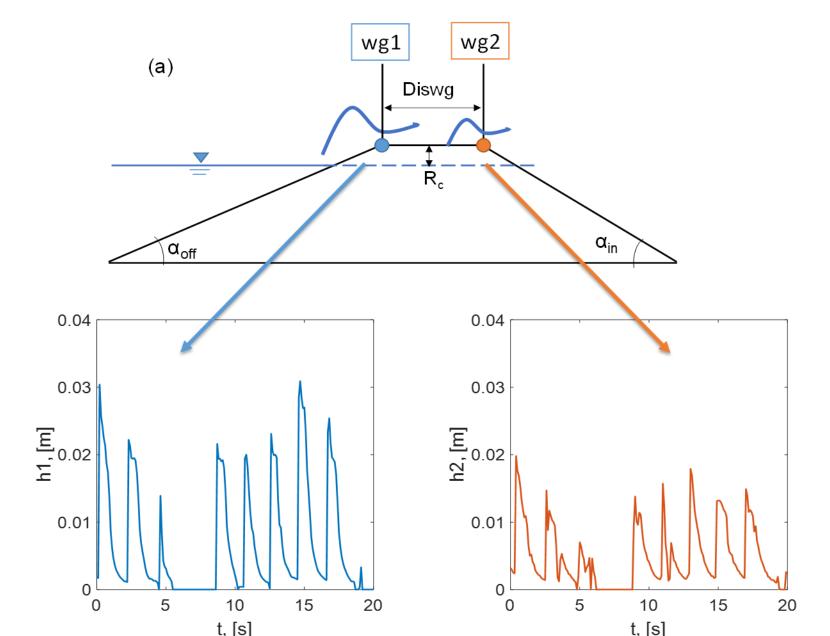
Outline

- Presentation and description of the new procedure
 - First algorithm: the wave identification
 - Second algorithm: the wave coupling
- Benefits and limits of the procedure
 - Rare and frequent overtopping
 - Permeable structures and percolation
- Verification of the procedure
- Applications of the procedure
 - Determination of the extreme wave overtopping volumes
 - Wave celerities
- Conclusions





Scheme and parameters







1° step of the procedure: wave identification

Input 1° step: h₁, h₂ [m]

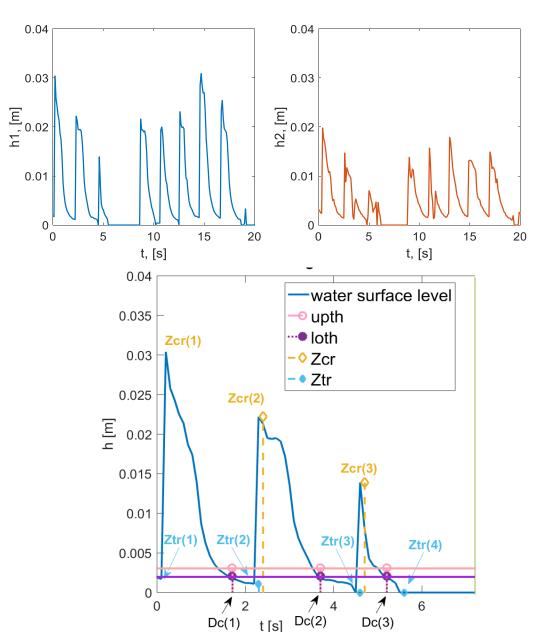


Definition of the 2 thresholds based on: H_s, R_c and diswg



Output 1° step

- Dc₁, Icr₁, Itr₁ [s] and Zcr₁, Ztr₁ [m]
- Dc₂, Icr₂, Itr₂ [s] and Zcr₂, Ztr₂ [m]



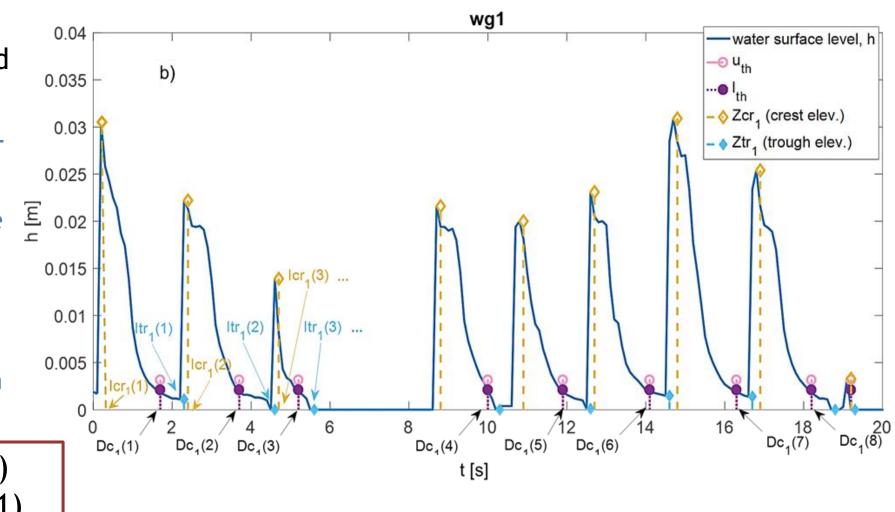


Wave identification: the outputs

- The outputs of the 1st step are the time ordered sequences of:
 - the instants of zero-downcrossing (Dc [s])
 - the instants of occurrence of the wave crests and troughs (lcr, ltr [s])
 - the wave crest and trough heights (Zcr, Ztr [m])

$$H(i)=Zcr(i)-Ztr(i)$$

 $T(i)=Dc(i)-Dc(i-1)$







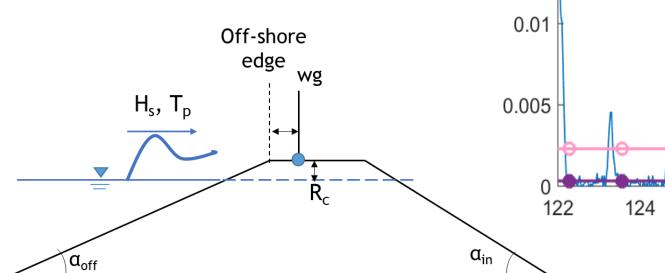


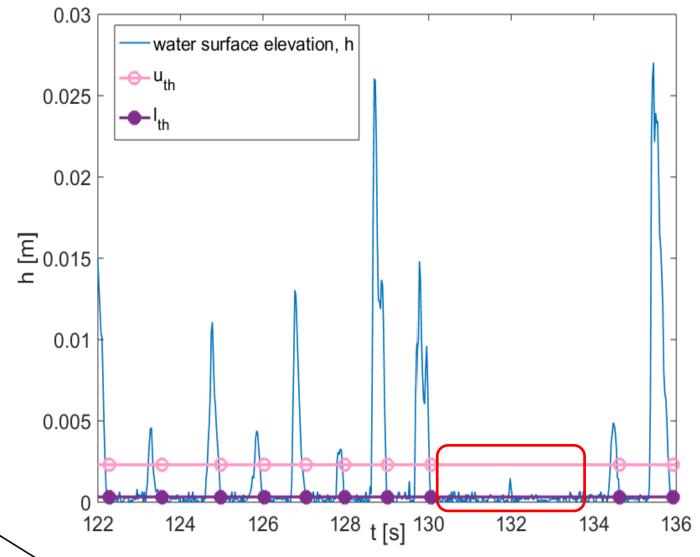




Wave identification: definition of the thresholds

- The thresholds vary with:
 - The structure freeboard (R_c)
 - The wave characteristics (H_s and T_p)
 - The distance of the wg from the dike off-shore edge

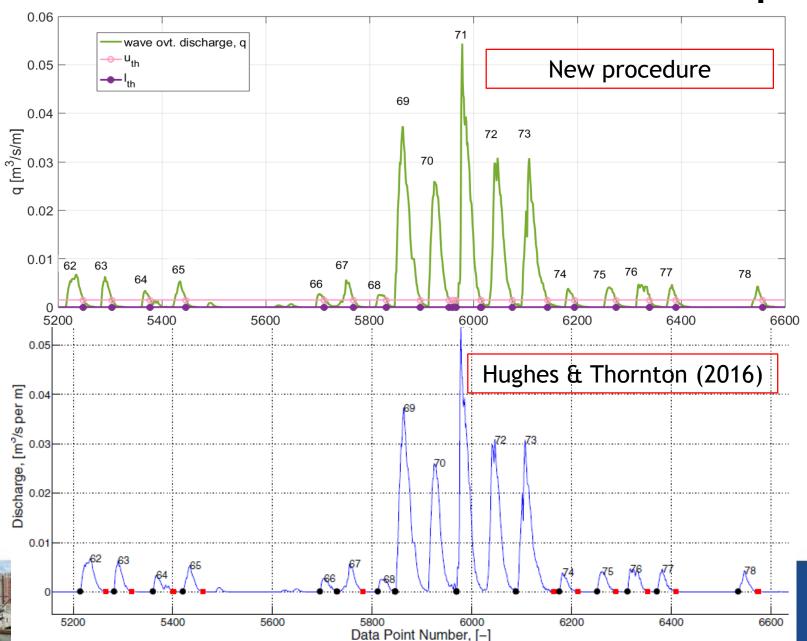








Verification of the identification step







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Verification of the identification step

Tests parameters				Hughes & Thornton procedure					New procedure		
Test	H _{m0}	T _{m-1,0}	Nw	Auto.	Man.	Total	Auto/Total	D	Auto.	Auto/Total	D
ID	[m]	[s]	[-]	V	V	V_{HT}	V_{HT}	P _{ovt}	V	V_{HT}	P _{ovt}
0198	0.103	1.619	1180	486	177	663	73%	56%	636	96%	54%
0199	0.094	1.164	1102	211	235	446	47%	40%	356	80%	31%
0200	0.15	1.96	1276	453	361	814	56%	64%	839	103%	66%
0201	0.148	1.379	1150	254	493	747	34%	65%	680	91%	59%
0451	0.09	1.555	1097	535	46	581	92%	53%	621	107%	57%
0453	0.122	1.663	1120	726	142	868	84%	78%	931	107%	83%
0456	0.157	1.936	1093	617	275	892	69%	82%	904	101%	83%
0457	0.141	1.373	1116	521	267	788	66%	71%	794	101%	71%















2° step of the procedure: wave coupling

Input 2° step:
Dc₁ [s] and Zcr₁ [m]
Dc₂ [s] and Zcr₂ [m]

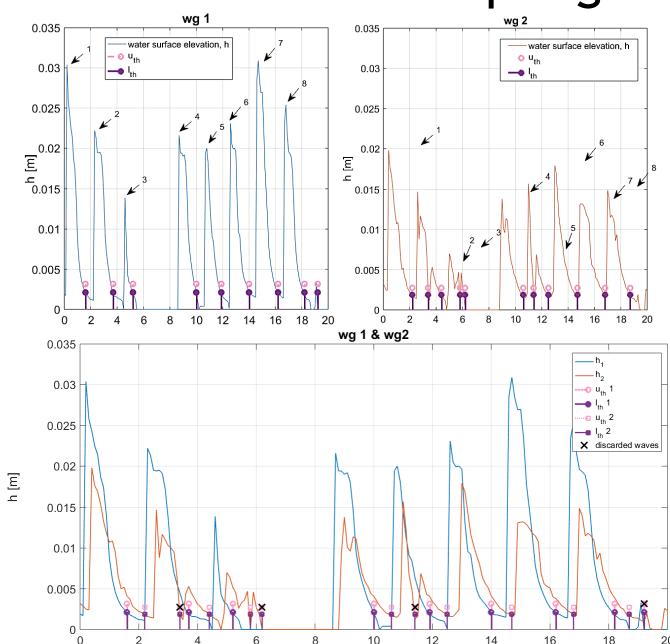


Definition of the maximum and minimum time lags based on:

H_s, T_D, R_c, diswg, sf



Output 2° step $Dc_{1s}, Dc_{2s} [s]$ $Zcr_{2s}, Zcr_{2s} [m]$ c [m/s]



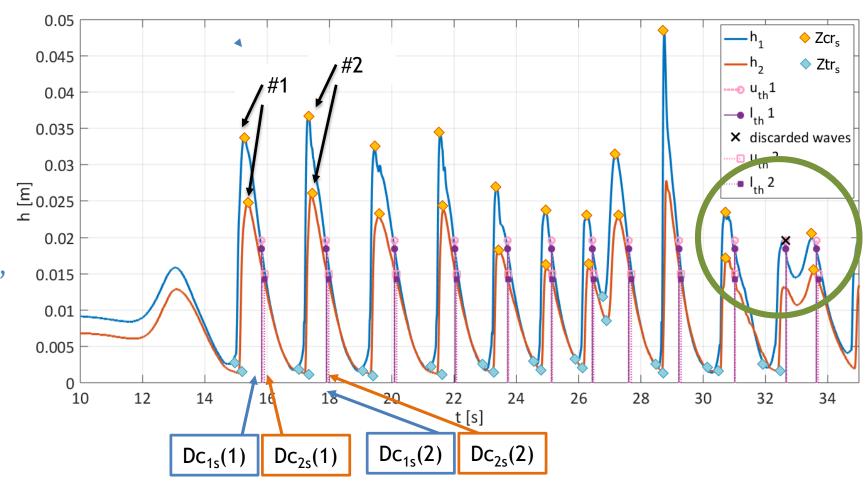




Wave coupling: the outputs

- The outputs of the 2nd step are the time ordered sequences of:
 - the instants of zerodown-crossing (Dc_{1s}, Dc_{2s} [s])
 - the instants of the wave crests and troughs (lcr_{1s}, ltr_{1s}, lcr_{2s}, ltr_{2s} [s])
 - the wave crest and trough heights (Zcr_{1s}, Ztr_{1s}, Zcr_{2s}, Ztr_{2s} [m])

of the coupled events at wg1 and wg2





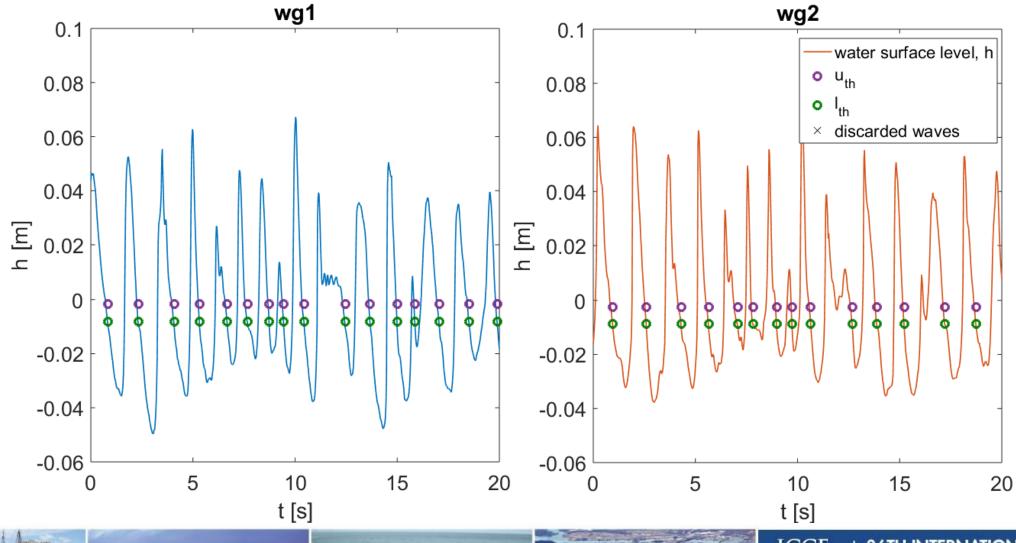








Frequent overtopping





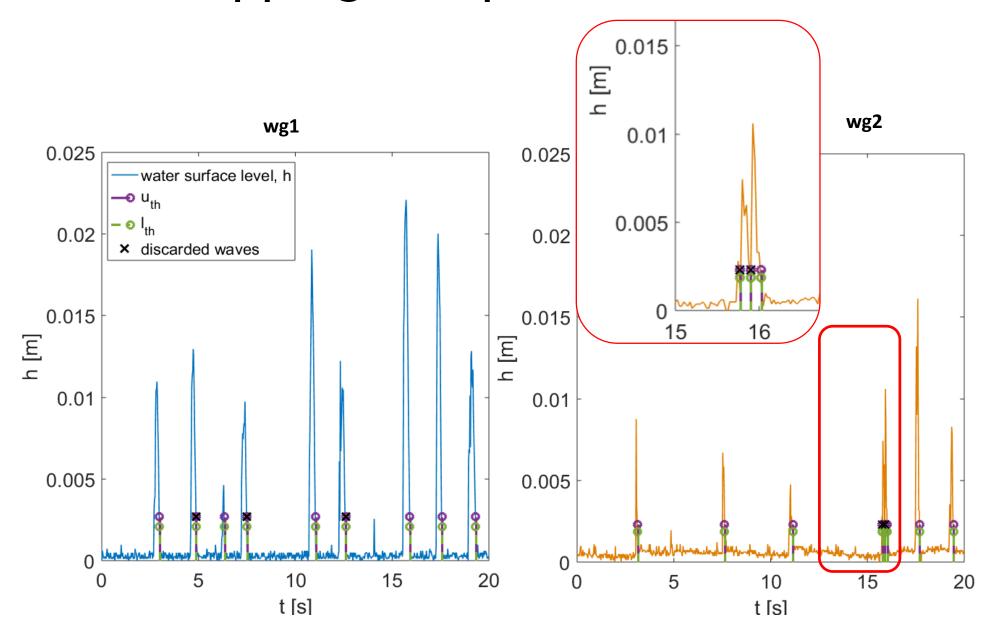








Rare overtopping and percolation



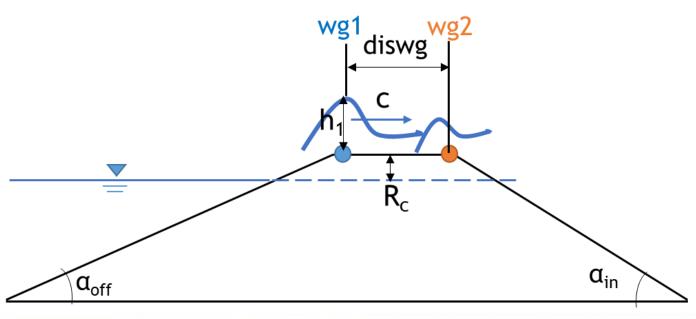




Wave coupling: definition of the time lags

- The max and min time lags vary with:
 - the distance between wg1 and wg2 (diswg, [m])
 - the celerity of the single waves
 (c, [m/s]); theoretically:
 c_{max} in deep water → dt_{min}
 c_{min} in shallow water → dt_{max}
 - the sample frequency of the signals (sf, [Hz])

$$\begin{cases} dt_{min} = max \left(\frac{diswg}{c_{dw}}; \frac{1}{sf} \right), & with c_{dw} = \frac{Lp}{Tp} \\ dt_{max} = \frac{diswg}{c_{sw}}, & with c_{sw} = min \left(\sqrt{gh_1} \right) \end{cases}$$













Verification of the coupling step

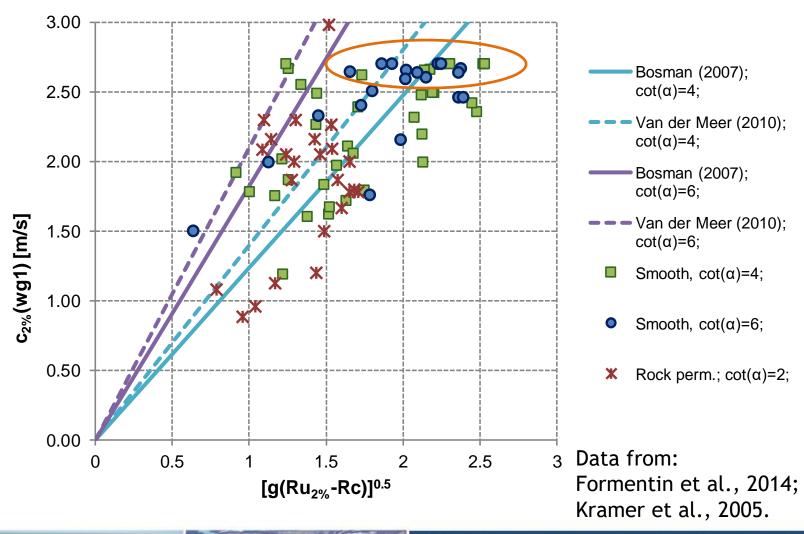
Determination of the wave celerities:

$$c(i) = \frac{diswg}{Dc2s(i) - Dc1s(i)}$$

- Comparison of the estimated c_{2%}-values with:
 - Bosman et al. (2007);
 - Van der Meer et al. (2010)

$$u_{2\%}(x_c) = c_u \cdot [(R_{u,2\%} - R_C)]^{0.5},$$

 $R_C \ge 0$











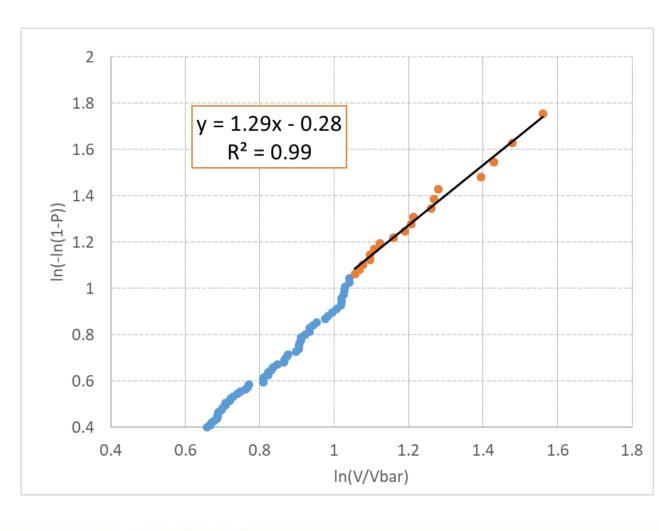


Applications: extreme overtopping volumes

 Determination of the Weibull's shape parameter b:

Shape parameter b.

$$P(V_{i} \ge \overline{V}) = \exp\left(-\left(\frac{V}{a}\right)^{b}\right)$$
0.25
0.15
0.05
0.05
830
835
840
845
850
855
860
865
870
875
880





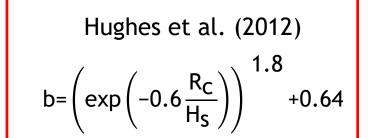


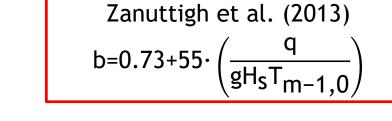


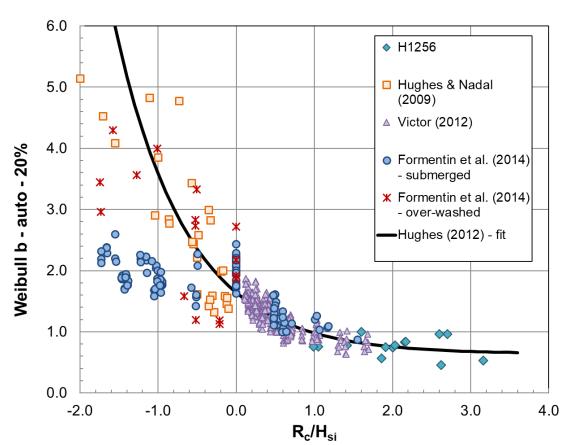


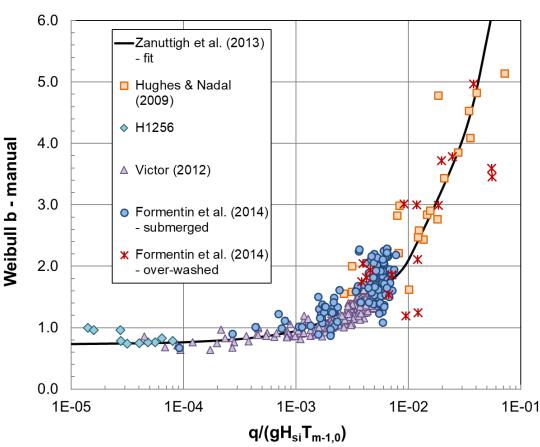


Applications: extreme overtopping volumes











Conclusions

- A new procedure has been developed for the identification and the coupling of the individual wave overtopping events
- The procedure is fully automatic and its accuracy is comparable to the accuracy of a manual analysis of the signals
- The procedure can process any kind of oscillatoriy signal in the time domain and can be applied to any structure type and structure emergence
- The wave identification algorithm can be applied to the processing of the discharge signals to reconstruct the distribution of the overtopping volumes
- The coupling algorithm allows the estimation of the wave celerities and the study
 of the evolution of the wave characteristics over the structure crest
- The main limit of the procedure is represented by the sample frequency of the input signals, i.e. a constraint independent of the procedure itself











References

- Bosman, G, J.W. Van der Meer, G. Hoffmans, H. Schüttrumpf and Verhagen., H.J., 2008. Individual overtopping events at dikes. ASCE, proc. ICCE 2008, Hamburg, Germany, p. 2944-2956.
- Formentin, S.M., Zanuttigh, B., van der Meer, J.W. and Lara, J.L., 2014. Overtopping flow characteristics at emerged and over-washed dikes, Proc. of XXXIV ICCE, Seoul (ROK).
- Kramer, M., Zanuttigh, B., van der Meer J. W., Vidal, C. and Gironella, X., 2005. Laboratory experiments on lowcrested structures, Coastal Engineering, 52, 867-88.
- Hughes, S.A., Thornton, C.I., van der Meer, J.W. and Scholl, B., 2012. Improvements in describing wave overtopping processes. ASCE, Proc. of XXXIII ICCE, Santander, Spain.
- Hughes, S.A. and Thornton, C.I. (2016). Estimation of time-varying discharge and cumulative volume in individual overtopping waves, Coastal Engineering, 117, 191-204.
- Van der Meer, J.W., B. Hardeman, G.J. Steendam, H. Schüttrumpf and H. Verheij, 2010. Flow depths and velocities at crest and inner slope of a dike, in theory and with the Wave Overtopping Simulator. ASCE, Proc. XXXII ICCE, Shanghai, China.
- Victor, L., 2012. Optimization of the Hydrodynamic Performance of Overtopping Wave Energy Converters: Experimental Study of Optimal Geometry and Probability Distribution of Overtopping Volumes. Phd, UGent, Zwijnaarde; Belgium.
- Zanuttigh, B., van der Meer, J.W., Bruce, T. and Hughes, S. (2013). Statistical characterisation of extreme overtopping wave volumes. Proc ICE, Coasts, Marine Structures and Breakwaters 2013, Edinburgh, UK.













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Thank you for your attention!







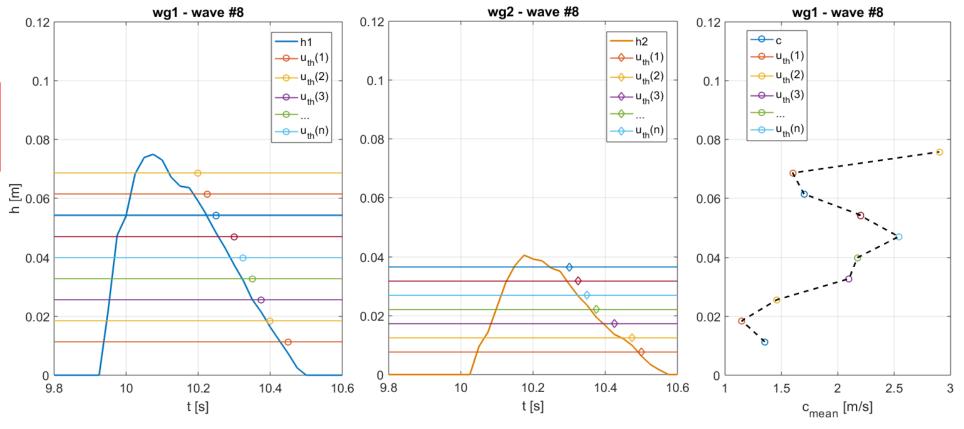


Wave coupling: the wave celerities

 For each i-th wave propagating from wg1 to wg2:

$$c(i) = \frac{diswg}{Dc2s(i) - Dc1s(i)}$$

 c can be computed at different elevations, reconstructing the instantaneous vertical profile of c







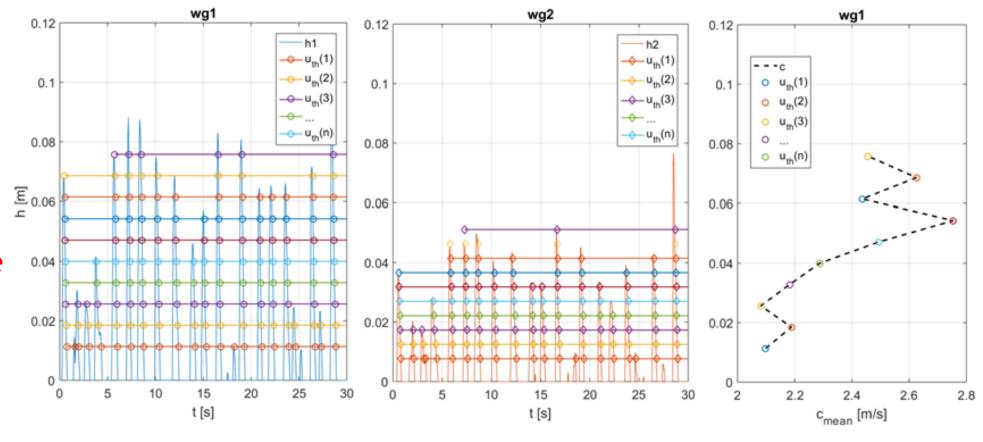






Wave coupling: the wave celerities

- Extraction of the statistics of the distribution of the c-values (mean, median, extreme quantiles, etc.)
- Reconstruction of the vertical profile of mean c













Applications: extreme overtopping volumes

		Hughes & Nadal (2009)	Experime ntal
$H_s/L_{m-1,0}$	0.02; 0.03; 0.04	0.02; 0.03; 0.04	0.03; 0.04
H _s [m]	2	2	0.8; 1.0; 1.2
h [m]	7.5; 8.5	7.5; 8.5	[5.2; 7]
R_{c} [m]	0; 0.5	0; 0.5	0; 0.5
R_c/H_s	-1.5; 1.5	-1.5; 1.5	[0.67; 1.8]
$cot(\alpha_{off})$	2; 3; 4	2; 3; 4	2; 4
G _c [m]	3; 6; 9	3; 6; 9	3; 6
#	91	91	113



