



# 36TH INTERNATIONAL CONFERENCE ON COASTAL ENGINEERING 2018

Baltimore, Maryland | July 30 – August 3, 2018

*The State of the Art and Science of Coastal Engineering*

## Geometric Characteristics Of Wave-Generated Sand Ripples A Full-Scale Experimental Study

**Dongxu Wang**

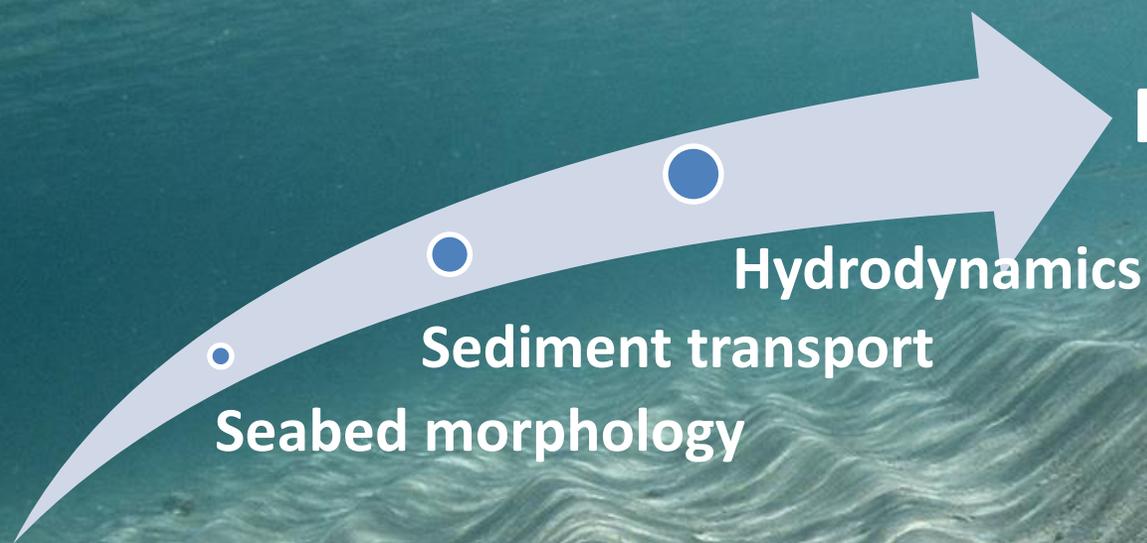
*Department of Civil and Environmental Engineering  
National University of Singapore*

**Jing Yuan, Dr.**

*Department of Civil and Environmental Engineering  
National University of Singapore*



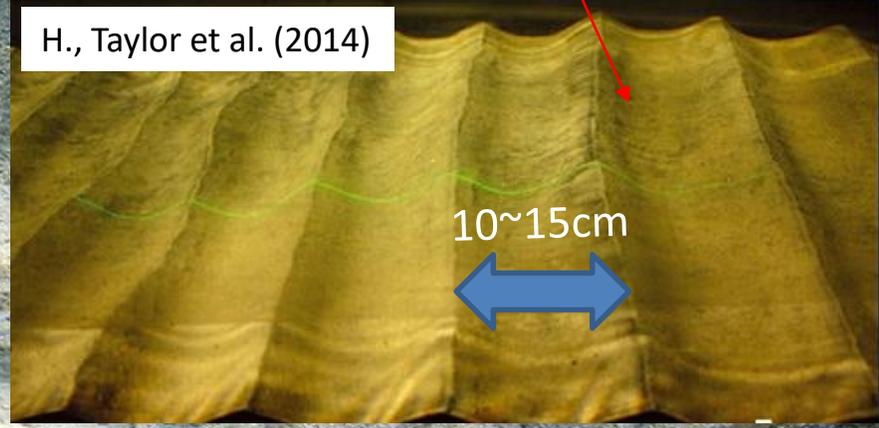
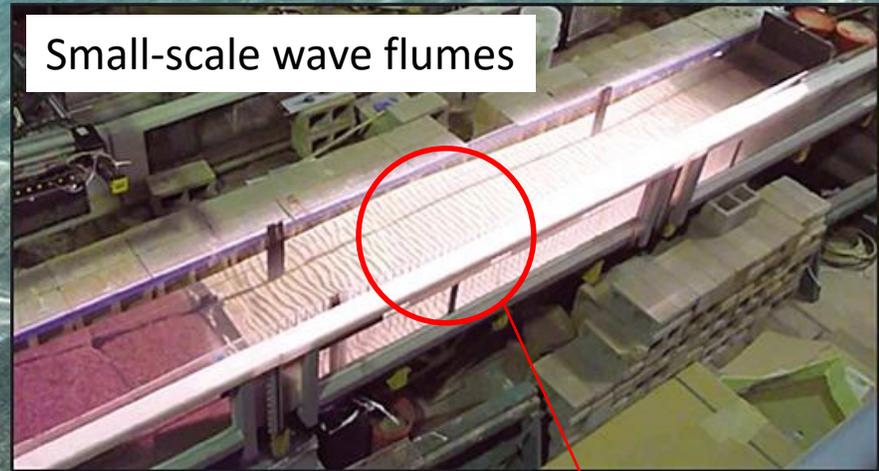
# Ripple shape



Seabed morphology  
Sediment transport  
Hydrodynamics

50~100cm

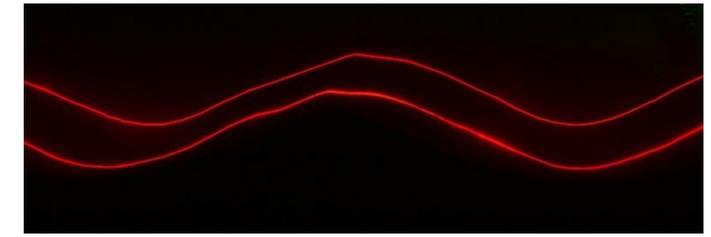
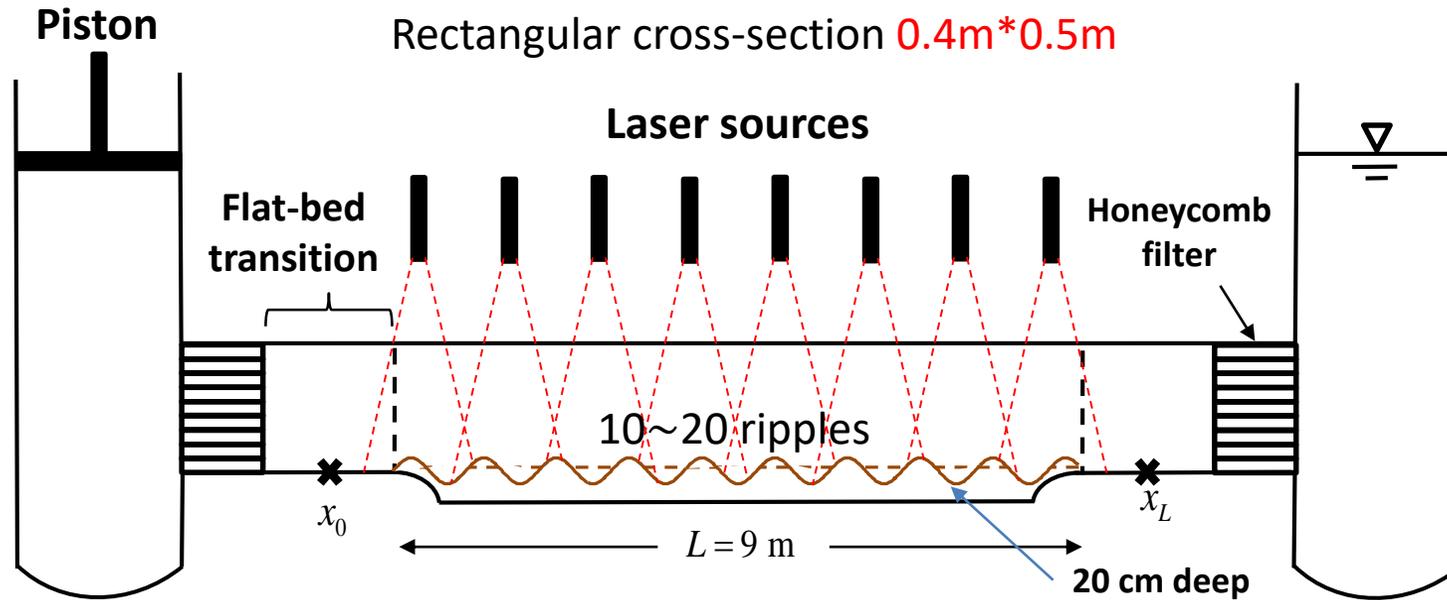
Are they similar?



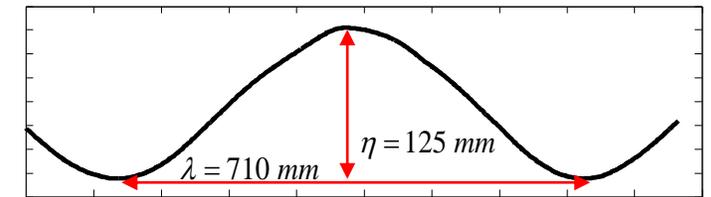
<https://fineartamerica.com/featured/underwater-sand-ripples-michael-szoenyi.html>



# Wave-Current-Sediment (WCS) Facility & Laser-based Bottom Profiler (LBP)



Laser line profile



Representative ripple profile

Well-sorted coarse sand    Sinusoidal oscillatory flows    Wave Reynolds number

$$d_{50} = 0.51\text{mm}$$

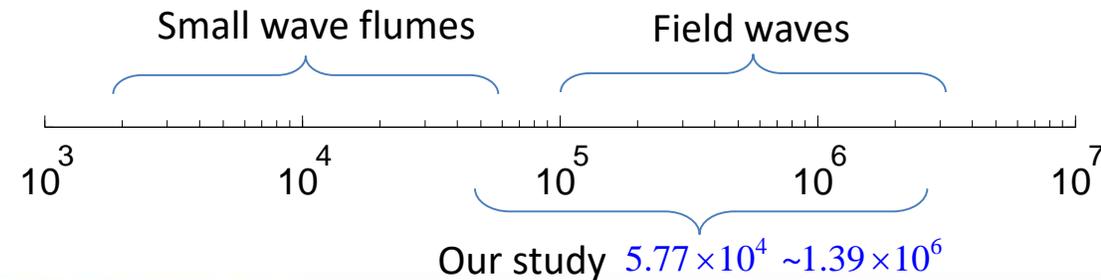
$$\psi_{cr} = 0.032$$

$$U_{\infty} = U_{bm} \cos \omega t$$

$$3.13\text{s} < T < 10.00\text{s}$$

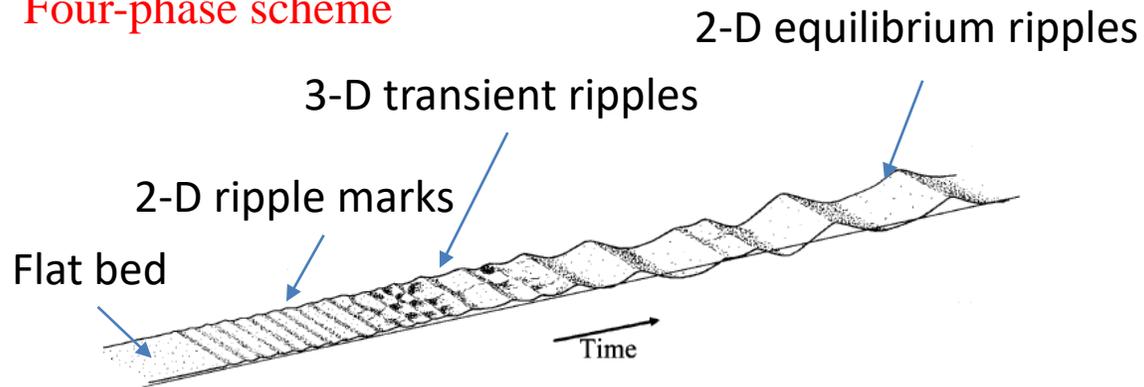
$$0.064 < \psi_{wmd} < 0.695$$

$$\text{Re}_w = \frac{U_{bm} \cdot A_{bm}}{\nu}$$

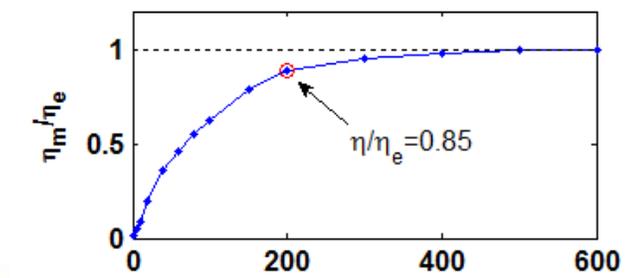
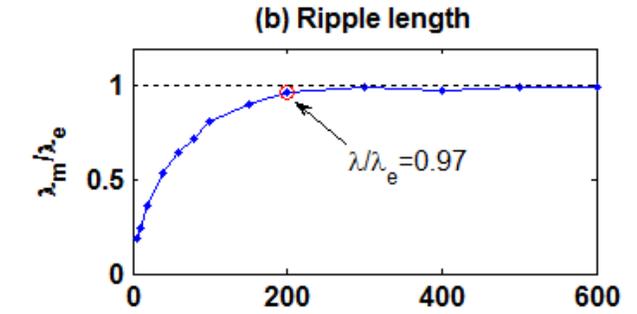
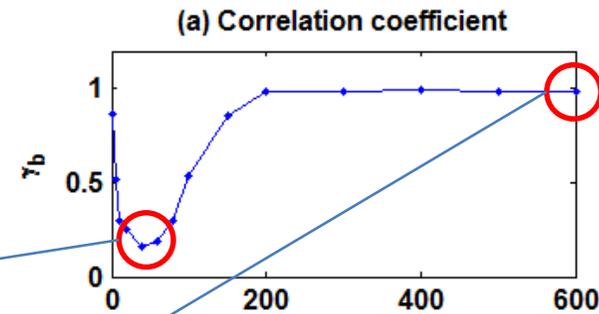
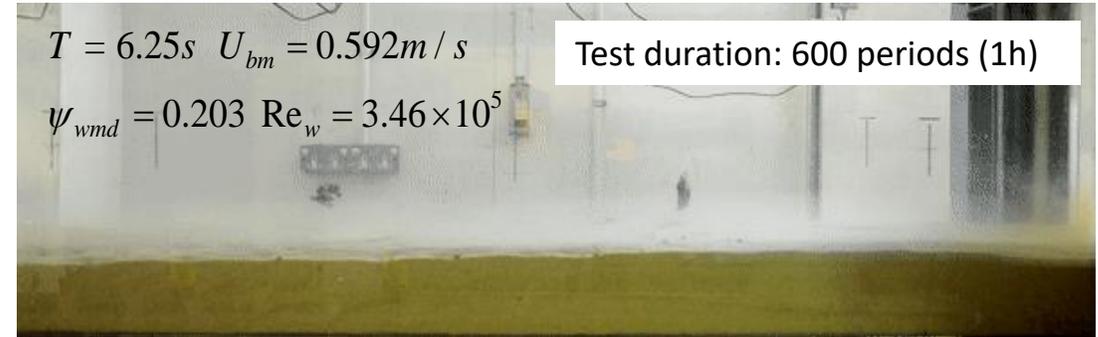
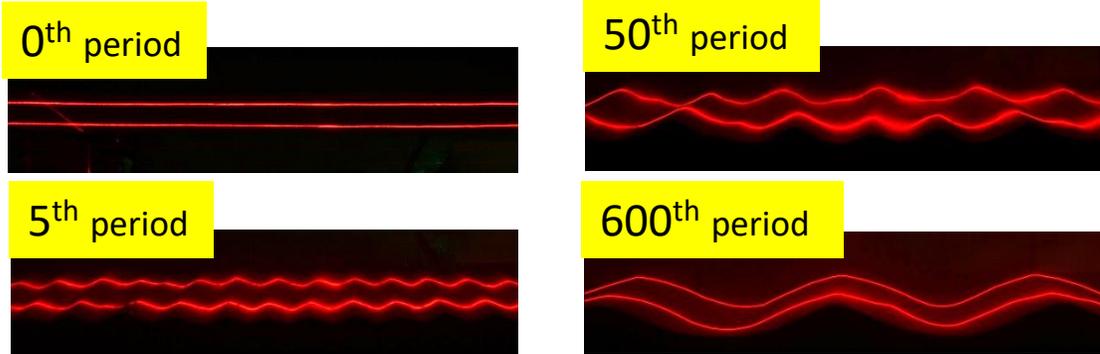


# 2-D Ripple development

## Four-phase scheme



Adopted from O'Donoghue, T. and G. S. Clubb (2001). "Sand ripples generated by regular oscillatory flow." Coastal Engineering 44(2): 101-115.



linear instability theory



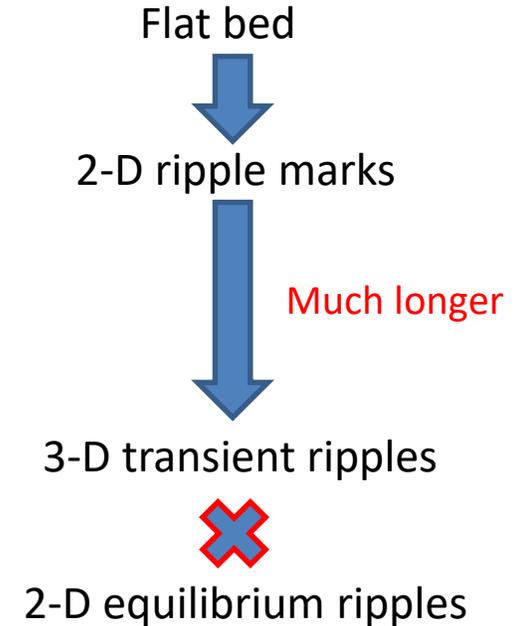
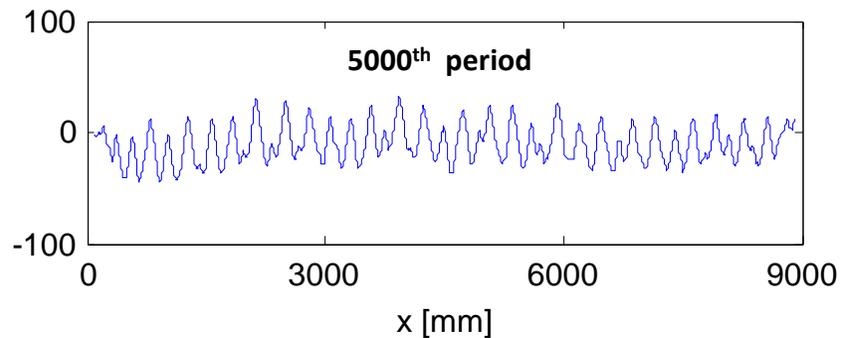
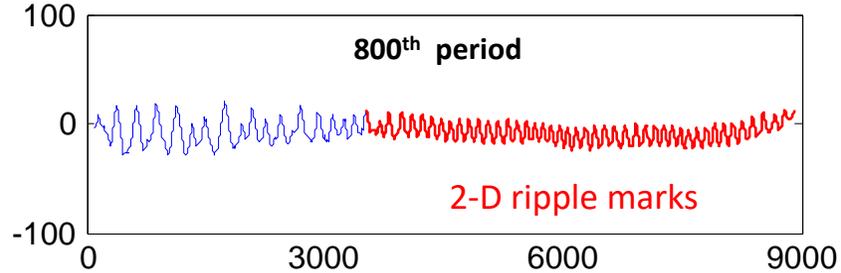
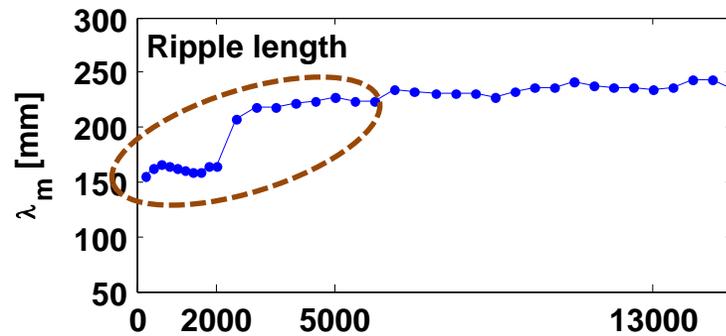
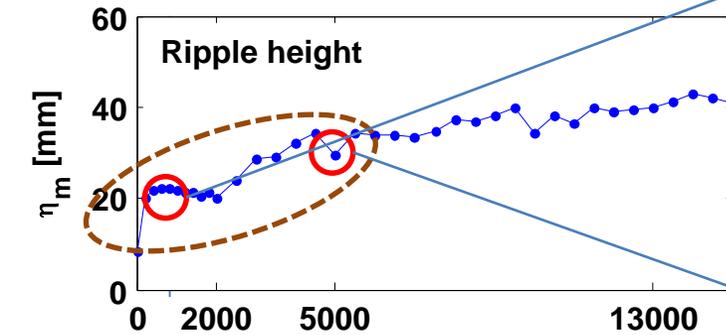
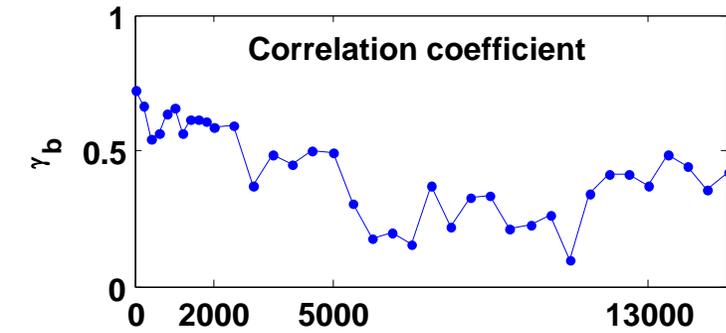
# A case of 3-D ripples: development

$$T = 4.17s \quad U_{bm} = 0.296m/s$$

$$\psi_{wmd} = 0.073 \quad Re_w = 5.77 \times 10^4$$

Flow conditions: Tb030

>13000 periods ~ **15 hours!**

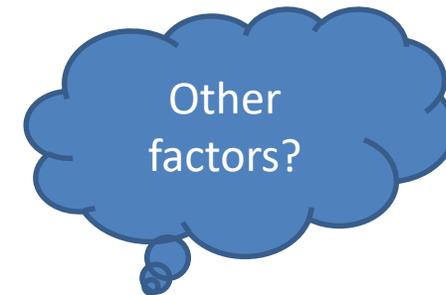
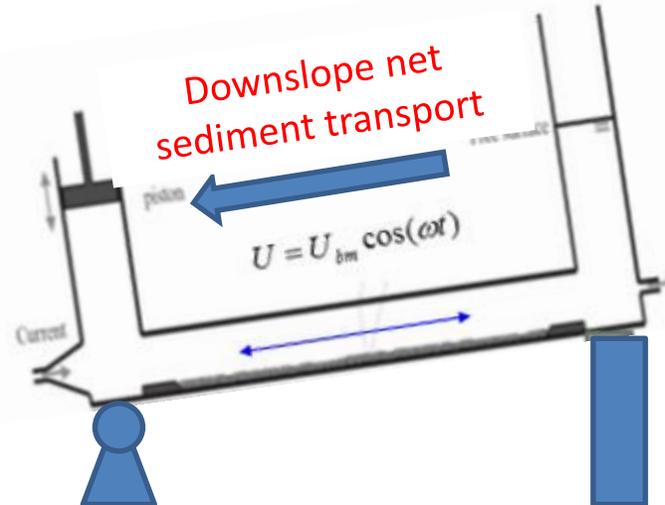
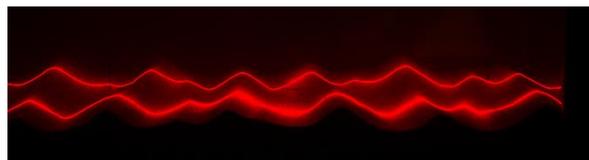
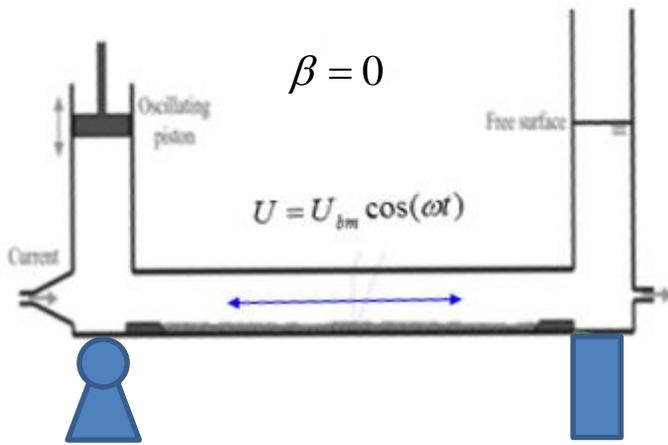
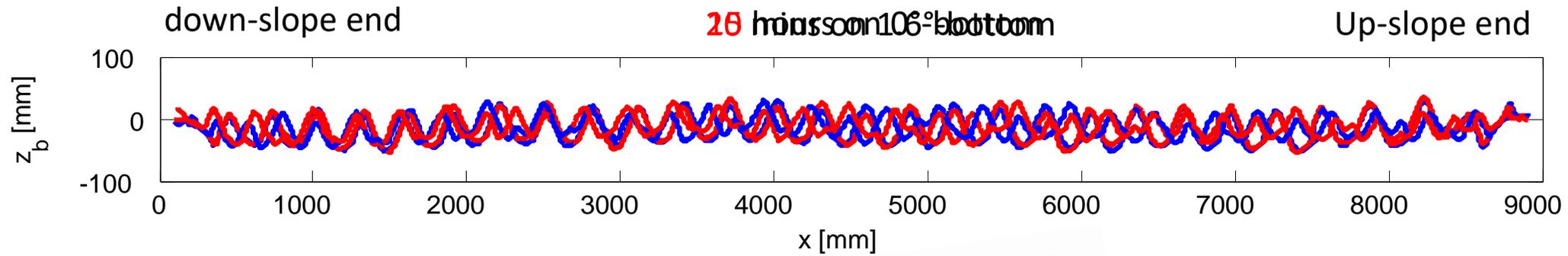


Other studies: **Coarse sand** and **weak flow conditions** → 2-D ripples



# A case of 3-D ripples: planform geometry

Flow conditions: Tb030  $T = 4.17\text{ s}$   $U_{bm} = 0.296\text{ m/s}$   $\psi_{wmd} = 0.073$   $Re_w = 5.77 \times 10^4$



Hypothesis: Net sediment transport rate affects the planform geometry



# 2-D Equilibrium Ripples: General Shape

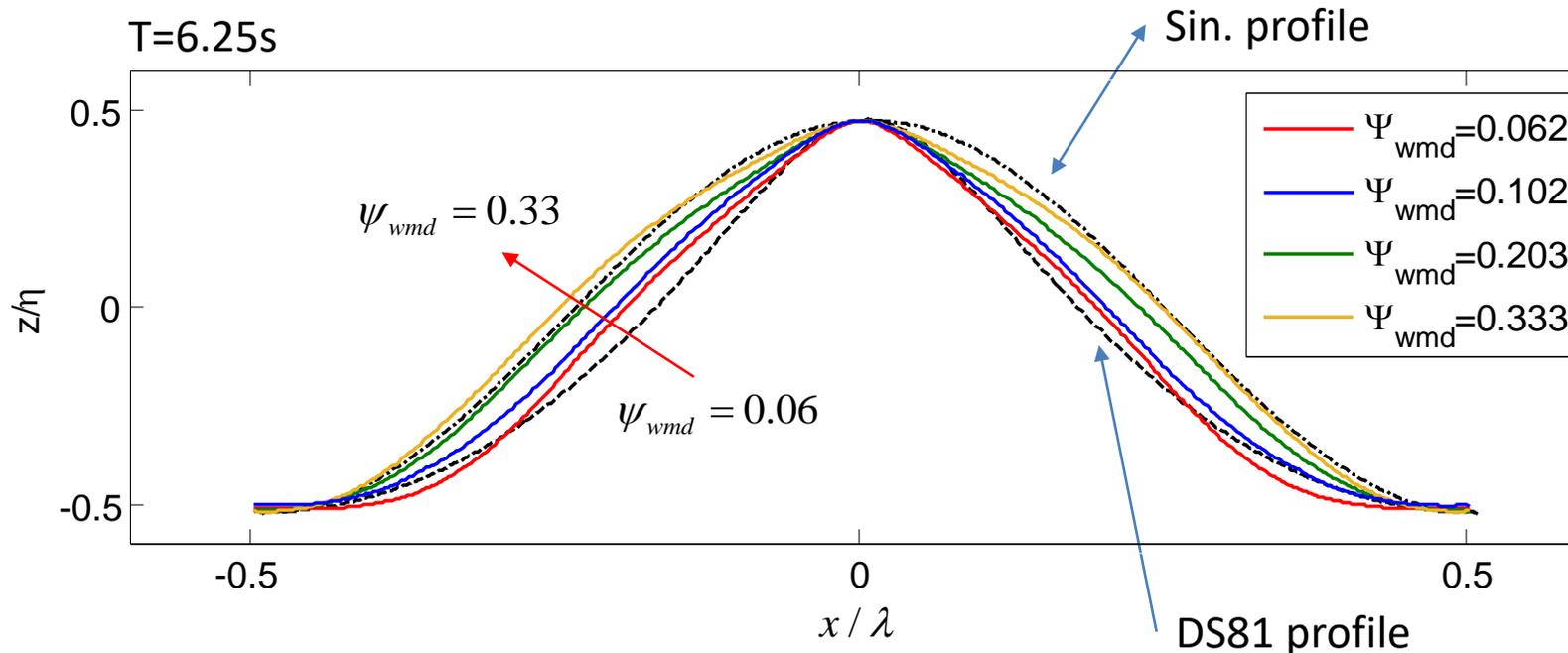
Sinusoidal profile  $z_* = \frac{1}{2} \cos(2\pi x_*) \quad |x_*| \leq \frac{1}{2}$

Theoretical profile proposed by Du Toit and Sleath (1981)

$$\begin{cases} x_* = \frac{\xi}{\lambda} - \frac{1}{2} \frac{\eta}{\lambda} \sin(2\pi x_*) \\ z_* = \frac{1}{2} \cos(2\pi \frac{\xi}{\lambda}) \end{cases} \quad \xi \leq \frac{\lambda}{2}$$

Based on the measurements with  $Re_w = 6.3 \times 10^3 \sim 5.6 \times 10^4$

close to **two-parabolas** profile but **rounded crest**



Not self-similar

Shields parameter is a controlling variable



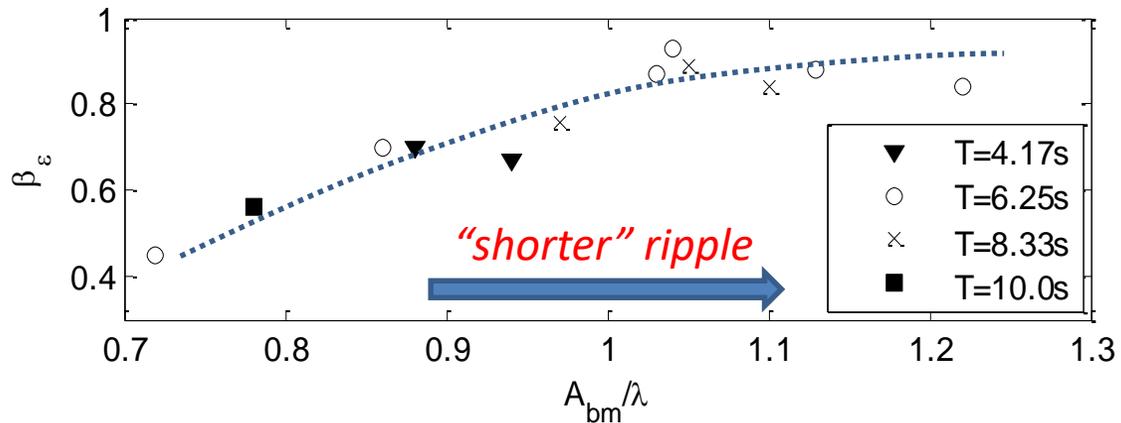
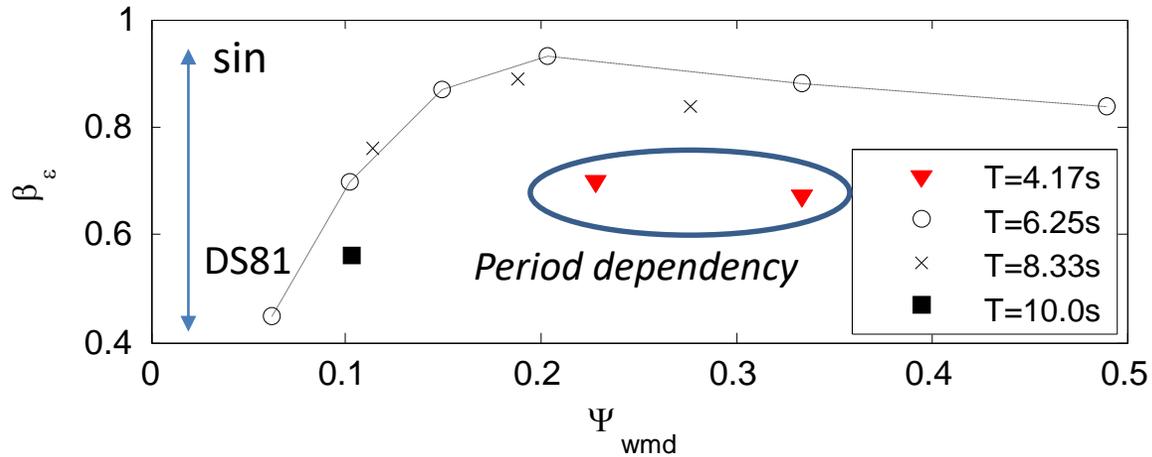
# 2-D Equilibrium Ripples: General Shape

Root-mean-square error

$$\varepsilon = \sqrt{\int_{-1/2}^{1/2} (z_{*,m}(x_*) - z_{*,l}(x_*))^2 dx}$$

Similarity index

$$\beta_\varepsilon = \frac{\varepsilon_{DS}}{\varepsilon_{DS} + \varepsilon_{\sin}}$$



Flow conditions: Ta030  $\psi_{wmd} = 0.062$   $Re_w = 8.66 \times 10^4$   $A_{bm} = 30 \text{ cm}$   $\lambda = 39.8 \text{ cm}$



Flow conditions: Ta080  $\psi_{wmd} = 0.333$   $Re_w = 6.16 \times 10^5$   $A_{bm} = 80 \text{ cm}$   $\lambda = 71 \text{ cm}$

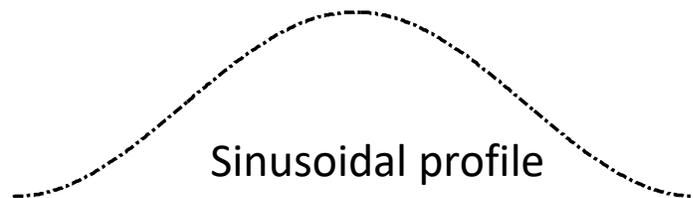
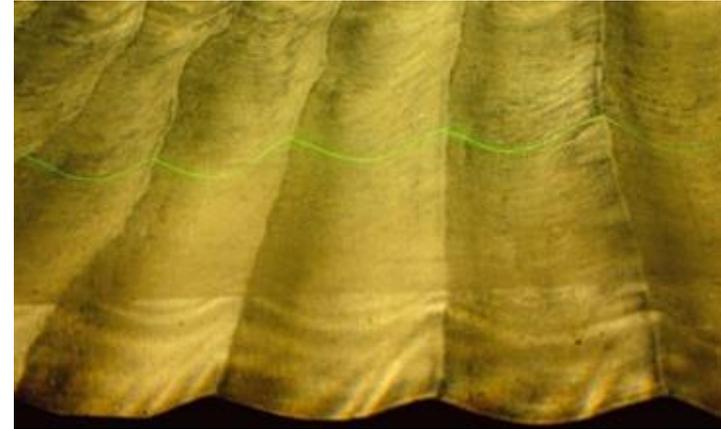


# 2-D Equilibrium Ripples: General Shape

Filed condition  
our OWT experiments (large  $Re_w$ )



Small wave flumes  
our OWT experiments (small  $Re_w$ )



Sinusoidal profile



DS81 profile

$T = 6-10$  s  
 $\Psi_{wmd} > 0.15$



Full-scale flow  
conditions  
( $Re_w \sim 10^5$ )



Larger  $A_{bm}/\lambda$



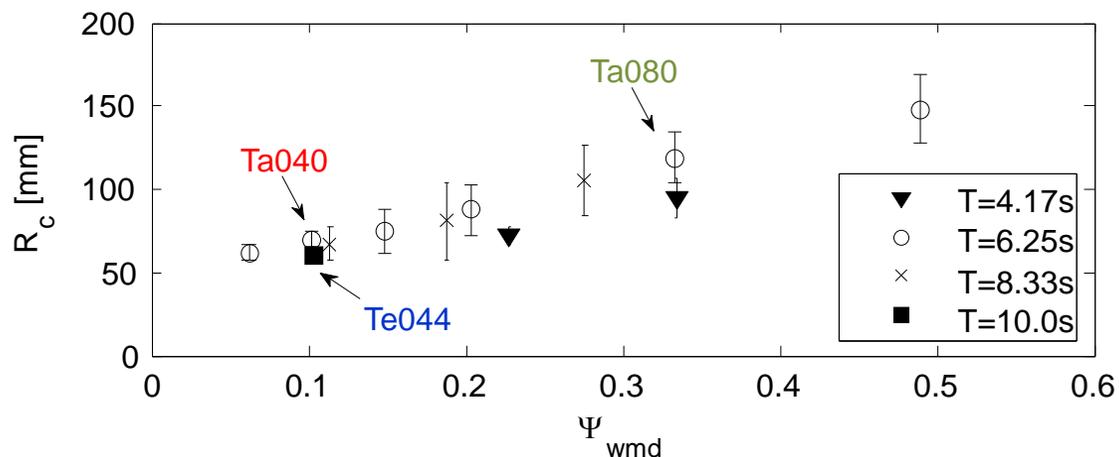
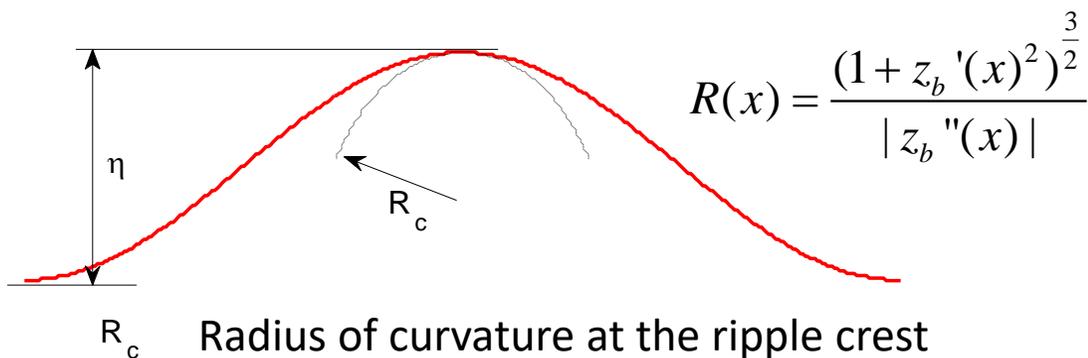
Vortex can affect larger  
area of ripple flank



**Sinusoidal**



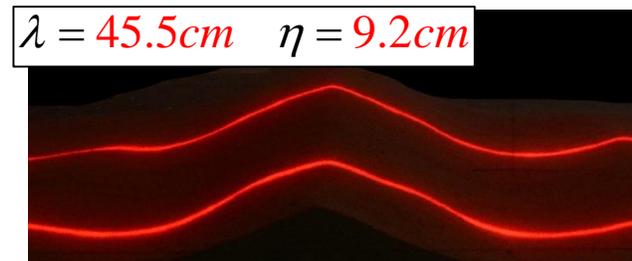
# What does the crest look like : Crest Roundness



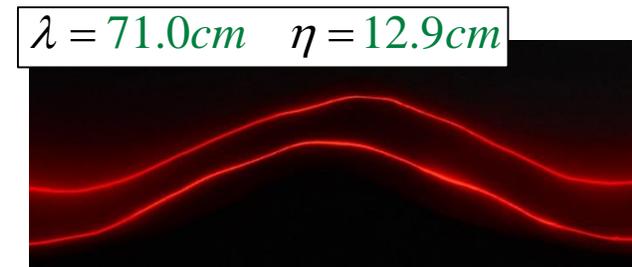
➤ larger Shields parameters larger  $R_c$

➤ Shields parameter  $\rightarrow$  roughness =  $d_{50}$

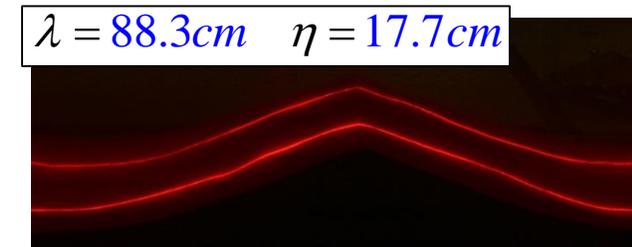
➤ Crest roundness is controlled by the **local flow conditions**



**Ta040**  $U_{bm} = 0.40\text{m/s}$   $T = 4.17\text{s}$   $\psi_{wmd} = 0.102$



**Ta080**  $U_{bm} = 0.80\text{m/s}$   $T = 6.25\text{s}$   $\psi_{wmd} = 0.333$



**Te044**  $U_{bm} = 0.44\text{m/s}$   $T = 10.00\text{s}$   $\psi_{wmd} = 0.103$



# Conclusion

- ❑ A **Full-scale** OWT experimental study on geometry of coarse-sand vortex ripples (ripple development and equilibrium ripple shapes).
- ❑ 3-D ripples become 2-D on sloping bed: **net sediment transport rate** also control the **planform geometry**.
- ❑ **Full-scale ( $Re_w \sim 10^5$ )** vortex ripples: **~ sinusoidal shape**.
- ❑ Flow with **larger shields parameter** tend to form **rounded-crest** ripples.





Thank you !

Q&A

WANG D. and YUAN J., (2018), "Bottom-slope-induced net sediment transport rate under oscillatory flows in the rippled-bed regime." *Journal of Geophysical Research: Oceans*(under minor revision)

WANG D. and YUAN J., (2018), "Geometric Characteristics of Coarse-sand Ripples Generated by Oscillatory Flows: A Full-Scale Experimental Study." *Coastal Engineering* (submitted)



**36TH INTERNATIONAL CONFERENCE  
ON COASTAL ENGINEERING 2018**  
Baltimore, Maryland | July 30 – August 3, 2018

# Planform Geometry

3-D ripples:

- Finer sand  $d_{50}$
- Longer excursion amplitude  $A_{bm}$
- Larger excursion velocity  $U_{bm}$

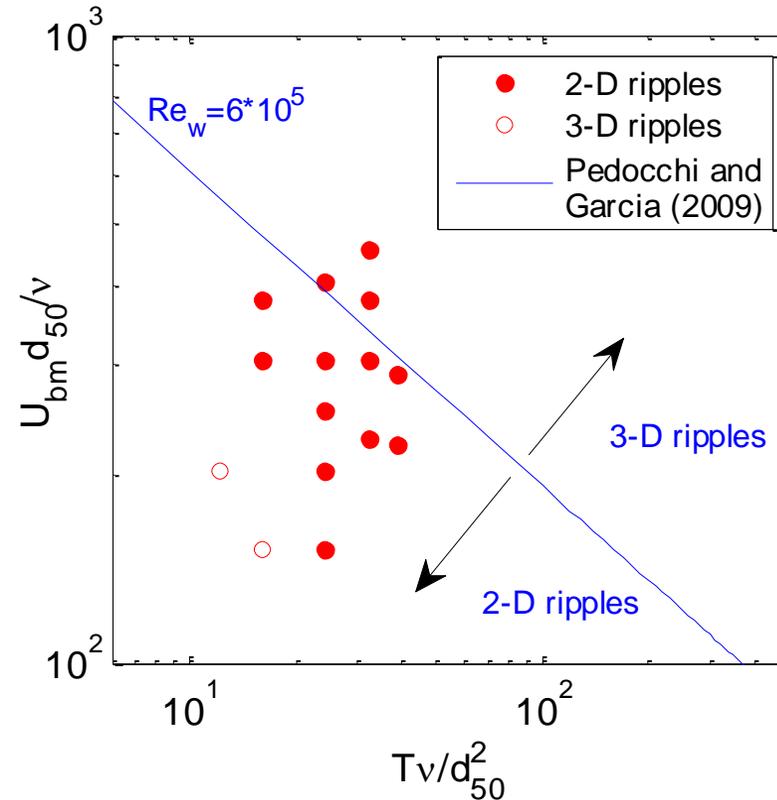
Pedocchi and Garcia (2009)

$$\frac{\sqrt{g(s-1)gd_{50}d_{50}}}{\nu} < 0.06 \sqrt{\frac{U_{bm}A_{bm}}{\nu}}$$



Small sand

Strong flow conditions



Tunnel width: 40 cm

Measured  $\lambda$  of 2-D ripples:  
44.7cm-90.4cm

$\lambda$  of 2-D ripples in the field:  
50cm-100cm  
e.g., Traykovski, Hay et al. (1999)

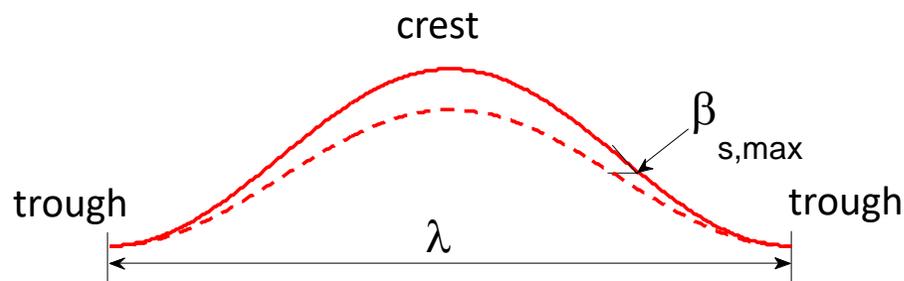
Our study: Given  $T$  Longer  $A_{bm}$  or larger  $U_{bm}$



2-D ripples

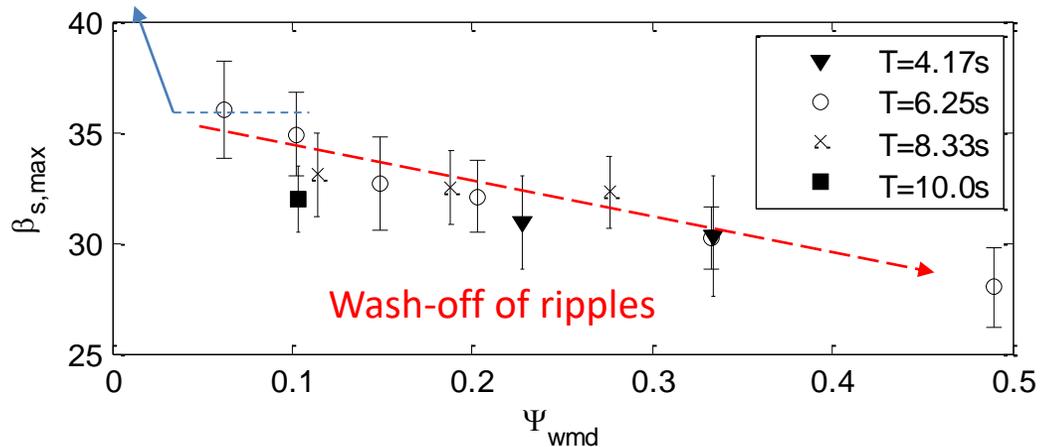


# What does the flank look like : Maximum Local Slope



$\beta_{s,max}$  Maximum slope between trough and crest

Angle of repose



weak suspension

deposit

Angle of repose

strong suspension

Flattens the ripples:  $\eta/\lambda \downarrow$

$\beta_{s,max}$  decreasing



36TH INTERNATIONAL CONFERENCE  
ON COASTAL ENGINEERING 2018  
Baltimore, Maryland | July 30 – August 3, 2018