



# 36TH INTERNATIONAL CONFERENCE ON COASTAL ENGINEERING 2018

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*The State of the Art and Science of Coastal Engineering*

## Total bottom shear stress for oscillatory flow over wave-generated sand ripples

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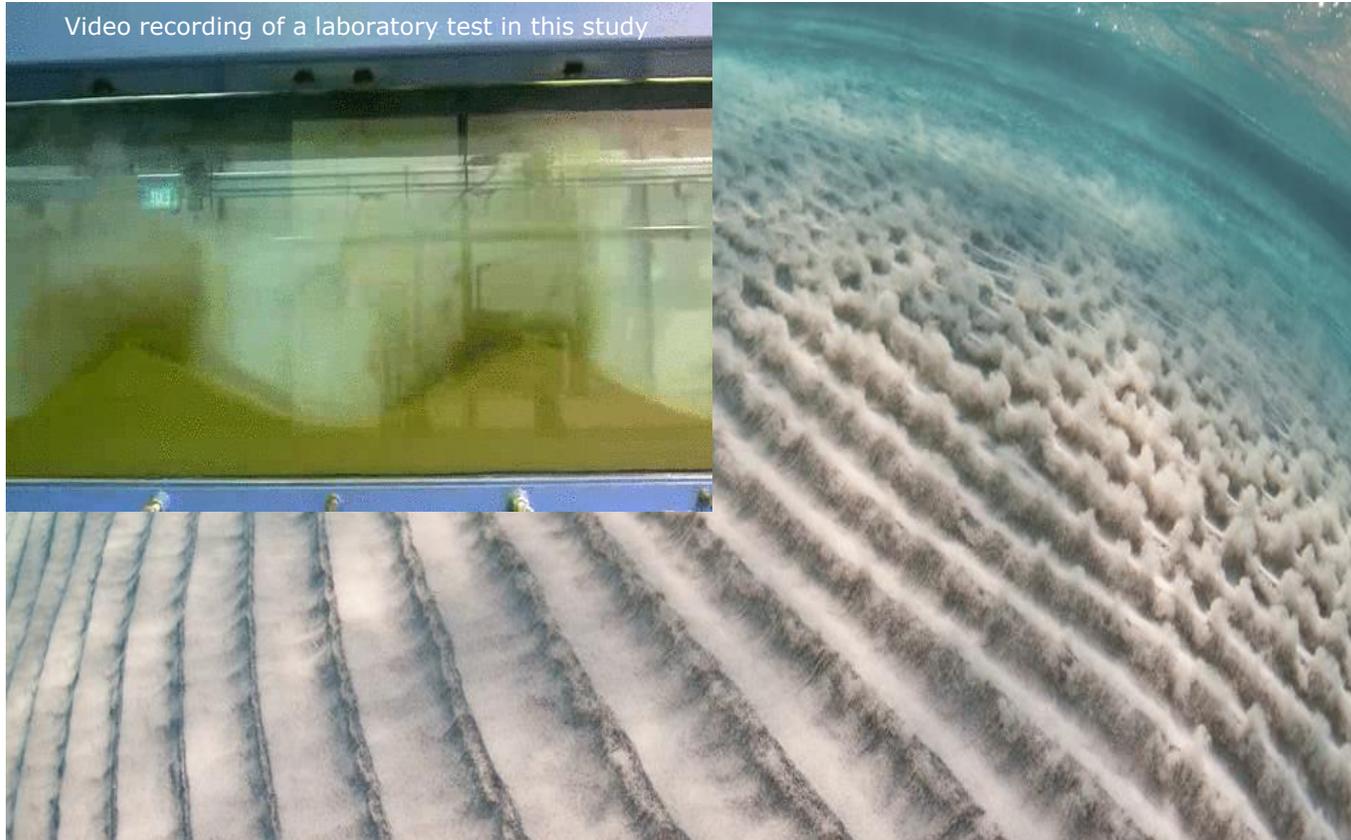
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# Motivation: wave-generated sand ripples



<http://www.naturephotoblog.com/2012/02/28/10356/alex-mustard>

**Coastal waves generate (vortex) ripples on a sandy seabed.**

**Coherent vortex motion is the dominant feature of local boundary layer flow**

**Ripples → enhanced flow resistance (bottom shear stress)**



# Form drag for oscillatory flows over vortex ripples

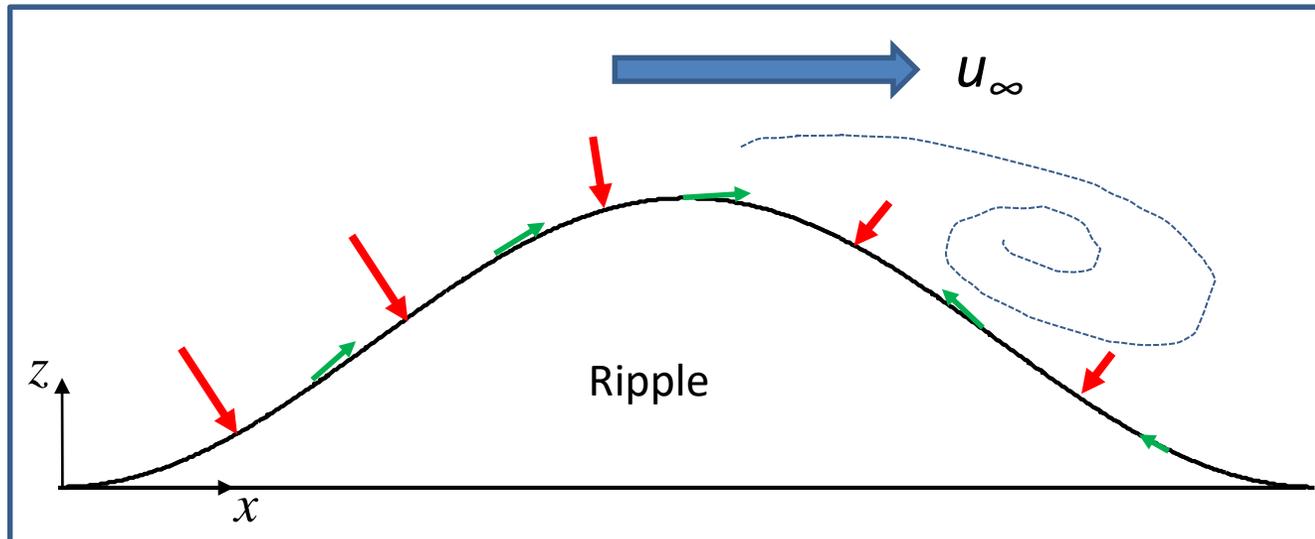
$$F_R = \int_0^\lambda \frac{p_\eta}{\rho} \frac{\partial \eta}{\partial x} dx + \int_0^\lambda \frac{\tau_\eta}{\rho} dx$$

Form drag

Skin friction

$$p_h = p - p_\infty,$$

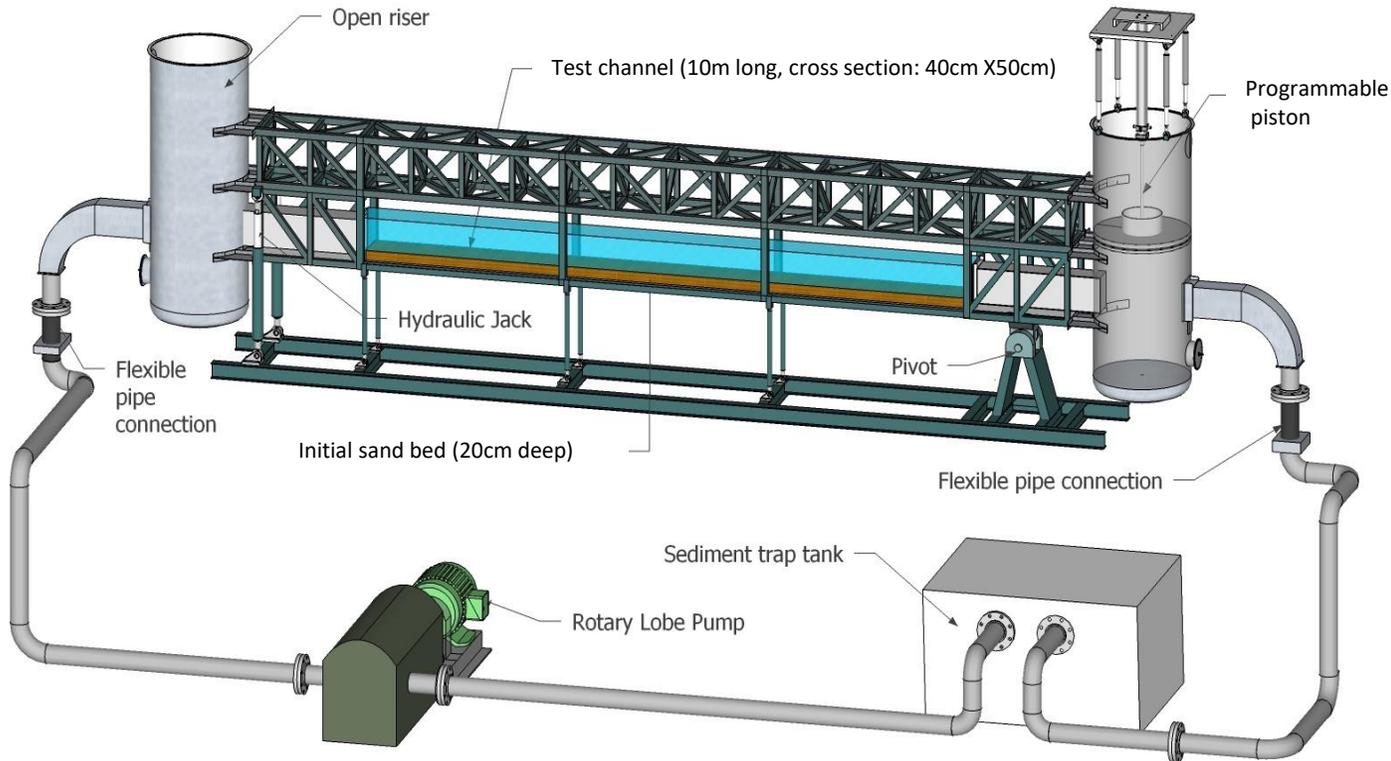
$$-\frac{1}{r} \frac{\partial p_\infty}{\partial x} = \frac{\partial u_\infty}{\partial t}$$



- ❖ Form drag is usually much larger than skin friction
- ❖ Important for understanding wave energy dissipation  $E_d$ , wave-current interaction, etc.
- ❖ Can be indirectly measured from  $E_d$ .
- ❖ Very little direct and full-scale measurements



# Research facility: oscillatory water tunnel



- 10 m-long test section with a 40cm-by-50 cm cross section
- Oscillatory flow with  $A_b < 2\text{m}$  and  $T > 3\text{s}$
- 20cm-deep movable bed (9 m long)
- Precise flow generation
- A laser-based bottom profiler for measuring ripple shape
- A PIV system for 2D flow measurements

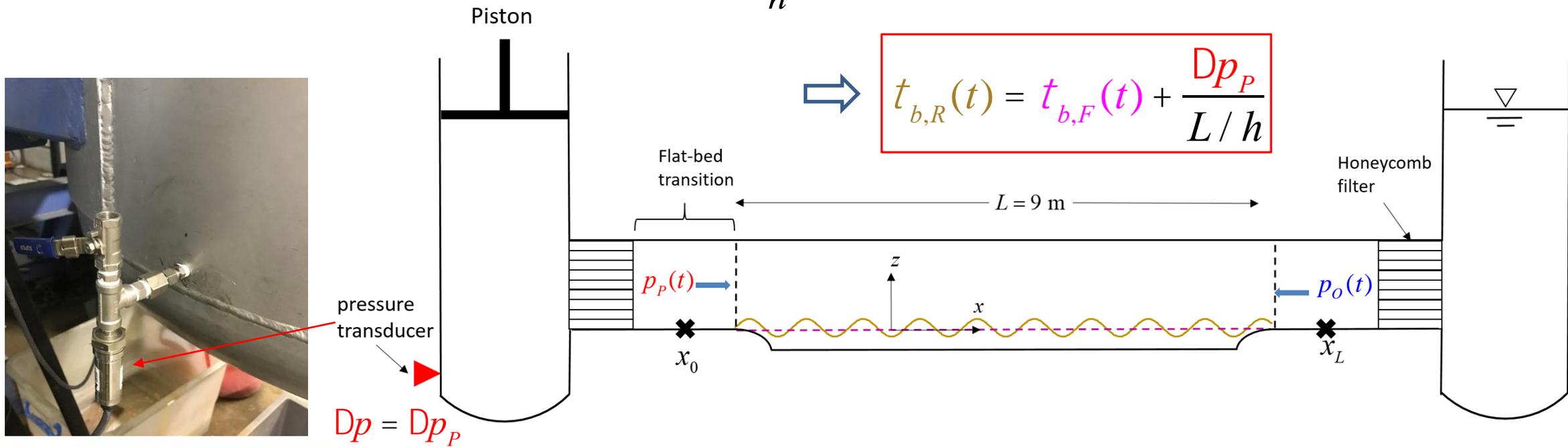


# A pressure-based measurement technique

- $p_p - p_o$  drives the oscillatory flow.
- $p_p$  increases as bottom shear stress increases.
- $p_o$  only depends on flow condition.
- For a pair of tests (**flat bed** vs. **rippled bed**) the difference in  $p_p$  is proportional to change in total bottom shear stress
- Same pressure difference inside the piston-end riser

$$\frac{L}{h} [t_{b,R}(t) - t_{b,F}(t)] = [p_P(t)]_R - [p_P(t)]_F = \Delta p_P$$

$$t_{b,R}(t) = t_{b,F}(t) + \frac{\Delta p_P}{L/h}$$



# Test conditions

$$u_{\nabla}(t) = U_{\nabla} \cos\left(\frac{2\pi}{T}t\right)$$

$$Re_w = \frac{A_b U_{\nabla}}{\nu}$$

Test ID	$A_b$ [m]	$T$ [s]	$\psi_{wmd}$	$U_{\infty}$ [m/s]	$H_R$ [mm]	$\lambda$ [mm]	No. of ripples	duration [h]	$Re_w$
Ta030	0.30	6.25	0.064	0.302	77	396	21	4.0	$9.1 \cdot 10^4$
Ta040	0.40	6.25	0.105	0.402	92	456	18	3.2	$1.6 \cdot 10^5$
Ta050	0.50	6.25	0.153	0.503	96	479	17	1.4	$2.5 \cdot 10^5$
Ta060	0.60	6.25	0.21	0.603	109	581	14	1.0	$3.6 \cdot 10^5$
Ta080	0.80	6.25	0.344	0.804	127	689	12	0.5	$6.4 \cdot 10^5$
Ta100	1.00	6.25	0.506	1.005	119	803	11	0.3	$1.0 \cdot 10^6$
Tc045	0.60	8.33	0.118	0.452	124	607	13	1.1	$2.7 \cdot 10^5$
Tc060	0.80	8.33	0.194	0.603	142	742	12	0.8	$4.8 \cdot 10^5$
Tc075	1.00	8.33	0.285	0.754	178	998	9	0.3	$7.5 \cdot 10^5$
Td044	0.70	10	0.107	0.44	186	829	10	3.3	$3.1 \cdot 10^5$
Td057	0.90	10	0.165	0.565	207	1242	7	1.5	$5.1 \cdot 10^5$

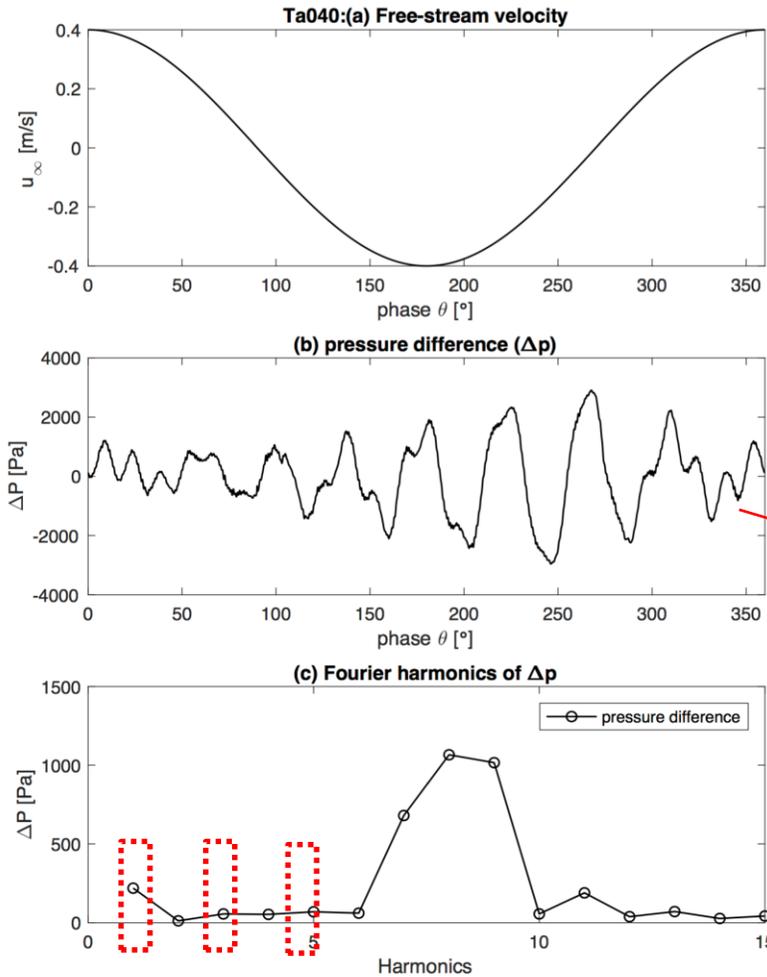
## Highlights:

- Coarse sand ( $d_{50}=0.51\text{mm}$ )
- All 2D equilibrium ripples
- 11 tests with
  - $Re_w=9.1 \cdot 10^4-7.5 \cdot 10^5$ .
  - $T=6.25-10$  s
  - $A_b=0.3-1.0$  m,  $U_{\infty}=0.3-1.0$  m/s
  - Ripple height  $H_R$ : 77-207 mm
  - Ripple length  $\lambda$ : 396-1242 mm



# Data correction

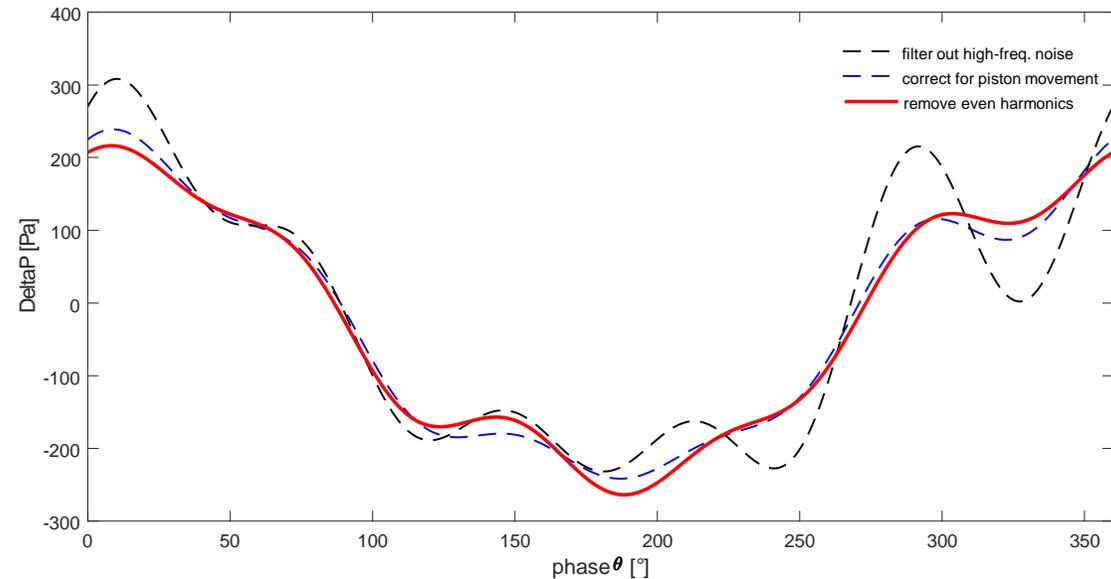
Example: Ta040 ( $U_{bm}=0.4\text{m/s}$ ,  $T=6.25\text{s}$ ,  $H_R=96\text{mm}$ ,  $\lambda=479\text{mm}$ )



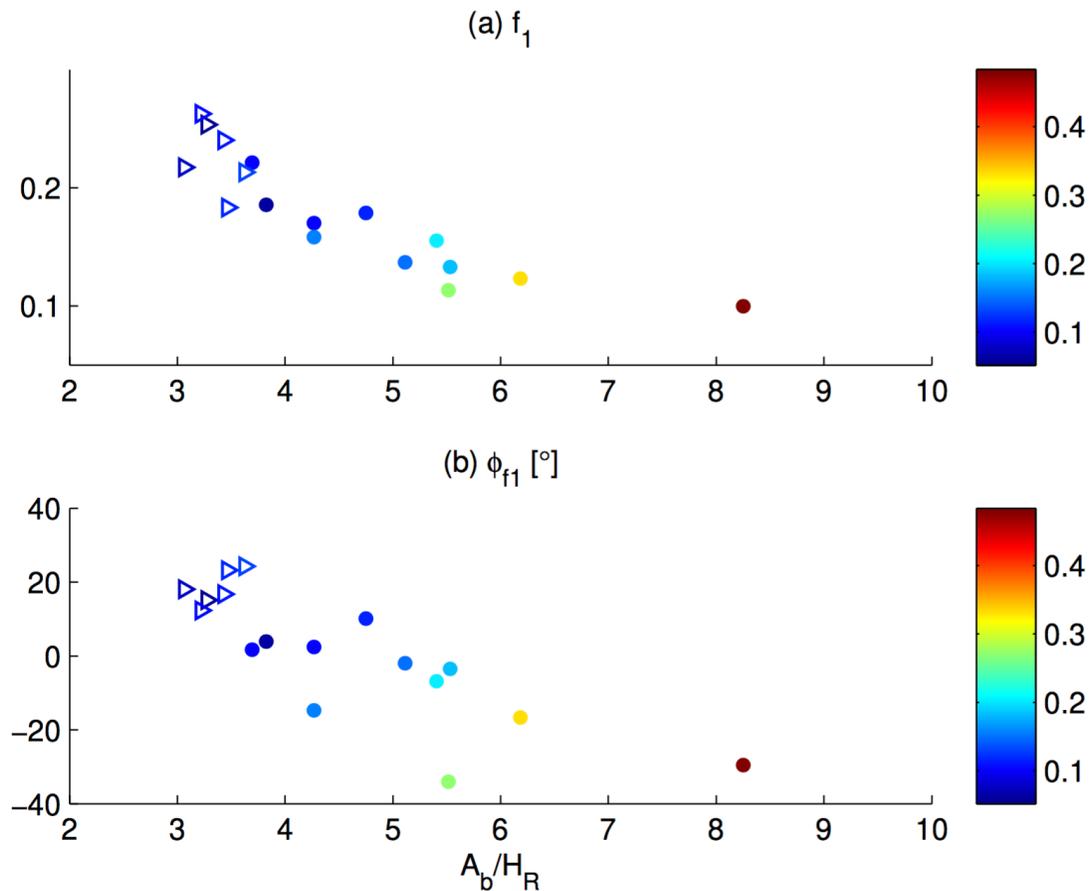
Correction:

- Remove high-freq. harmonics ( $>5^{\text{th}}$  harmonic)
- Correct for imperfect piston movement
- Remove even harmonics

**Only keep 1<sup>st</sup>, 3<sup>rd</sup> and 5<sup>th</sup> harmonics**



# First-harmonic total bottom shear stress



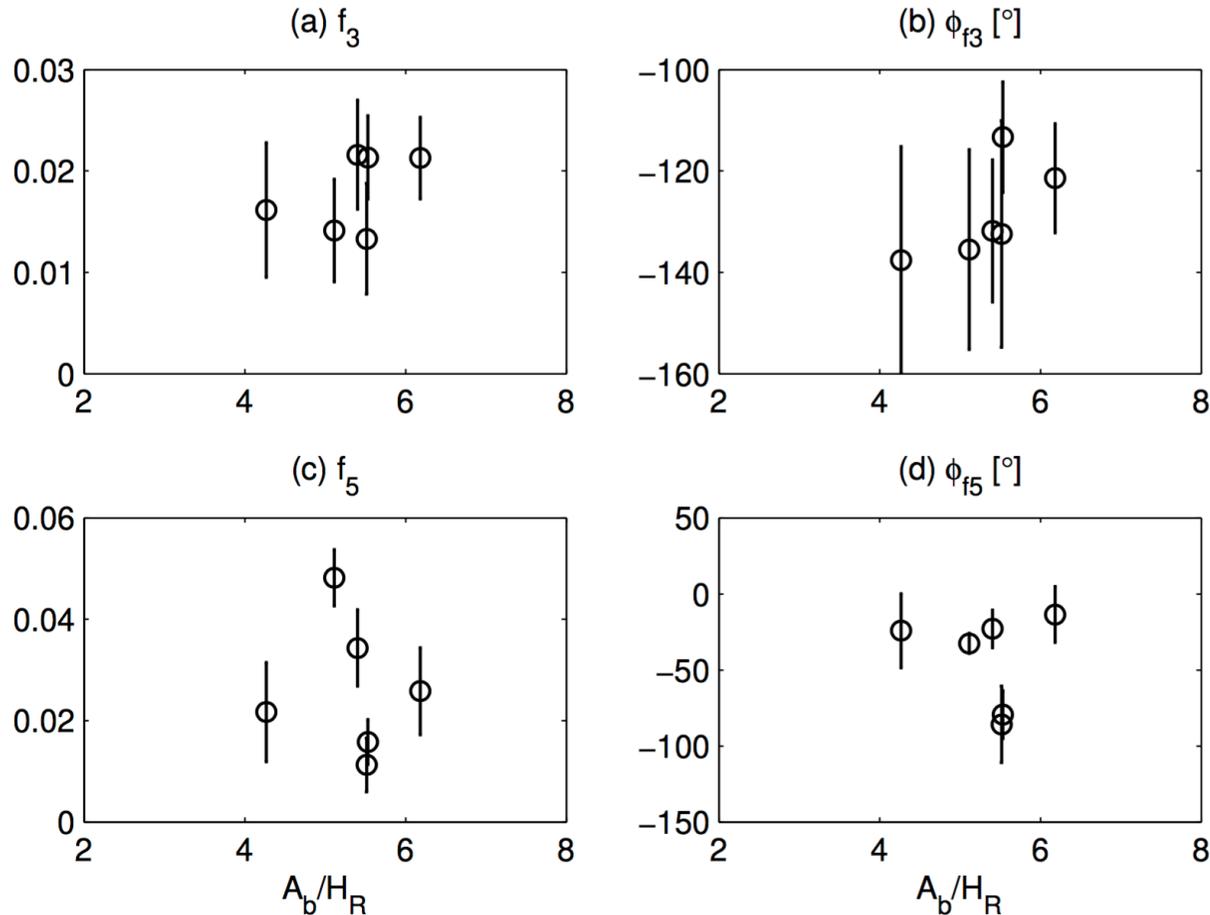
$$f_t = \frac{2t_b}{rU_\infty^2} = \text{Re} \left( \sum_{n=1,3,5} f^{(n)} e^{inWt} \right) = \sum_{n=1,3,5} f_n \cos(nWt + f_{fn})$$

- 1<sup>st</sup> harmonic is the dominant one
- Amplitude decreases with roughness or shields parameter: wash-off by strong flows
- In phase with free-stream velocity

$A_b/H_R$ : inverse of relative ripple height ( $\rightarrow \infty$  for flat bed)



# Higher-order harmonics

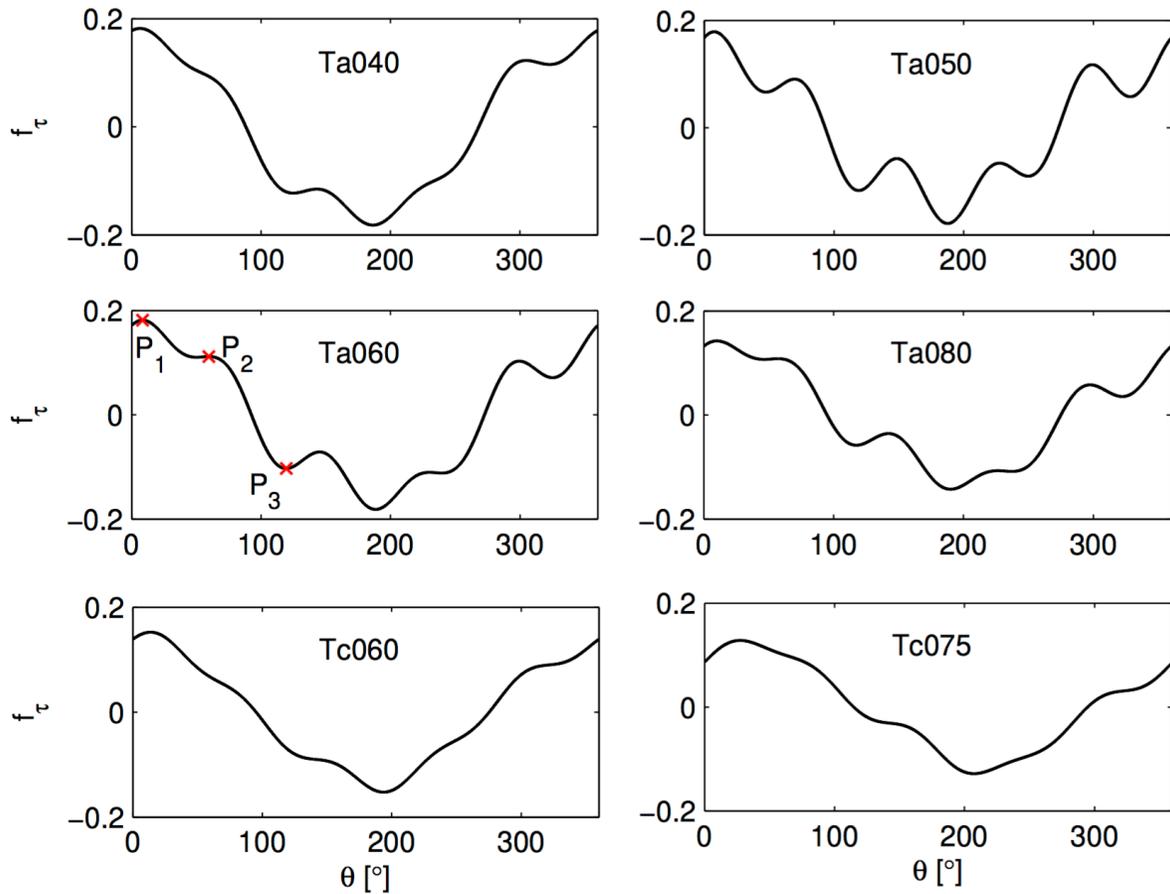


$$f_t = \frac{2t_b}{rU_\infty^2} = \text{Re} \left( \sum_{n=1,3,5} f^{(n)} e^{inWt} \right) = \sum_{n=1,3,5} f_n \cos(nWt + f_{fn})$$

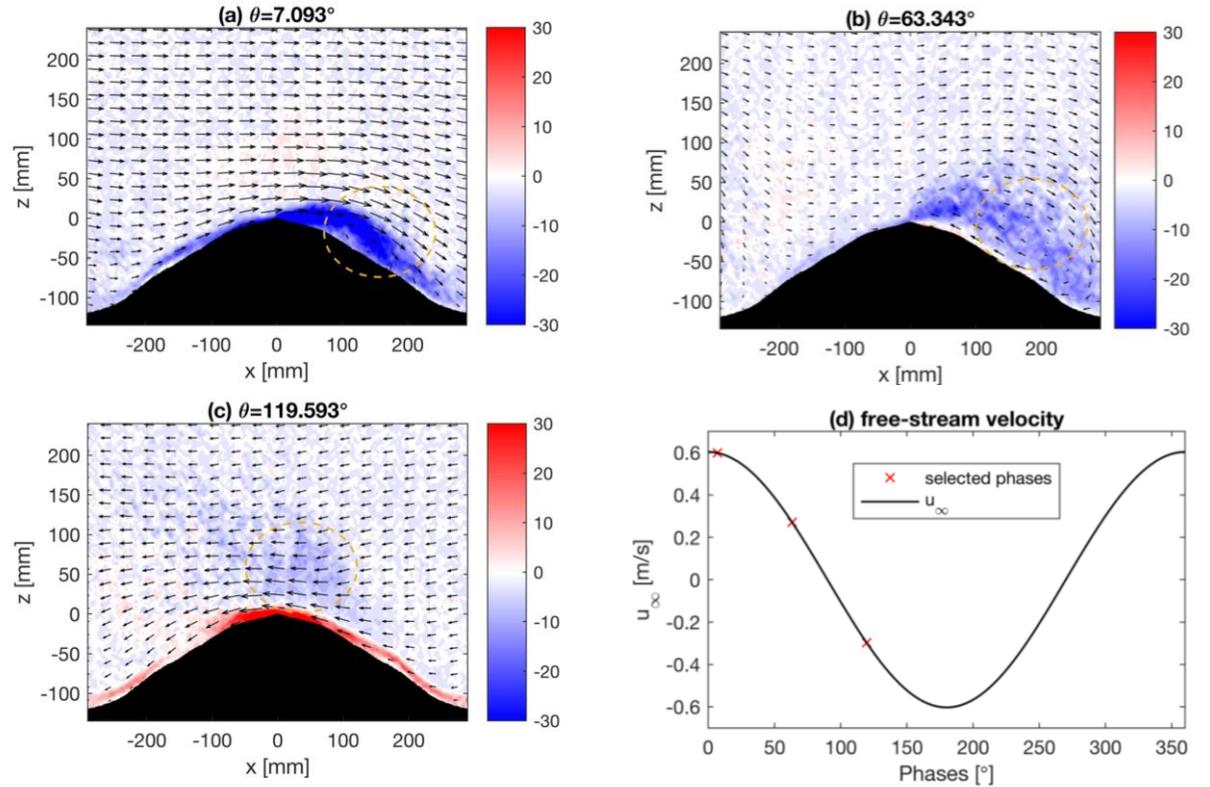
- 3<sup>rd</sup> and 5<sup>th</sup> harmonics suffer from large experimental error (6 of 11 tests are acceptable)
- Magnitude is of O(0.01), 5<sup>th</sup> harmonic is even larger than the 3<sup>rd</sup> harmonic
- No clear trend of variation



# Intra-period variation of total bottom shear stress



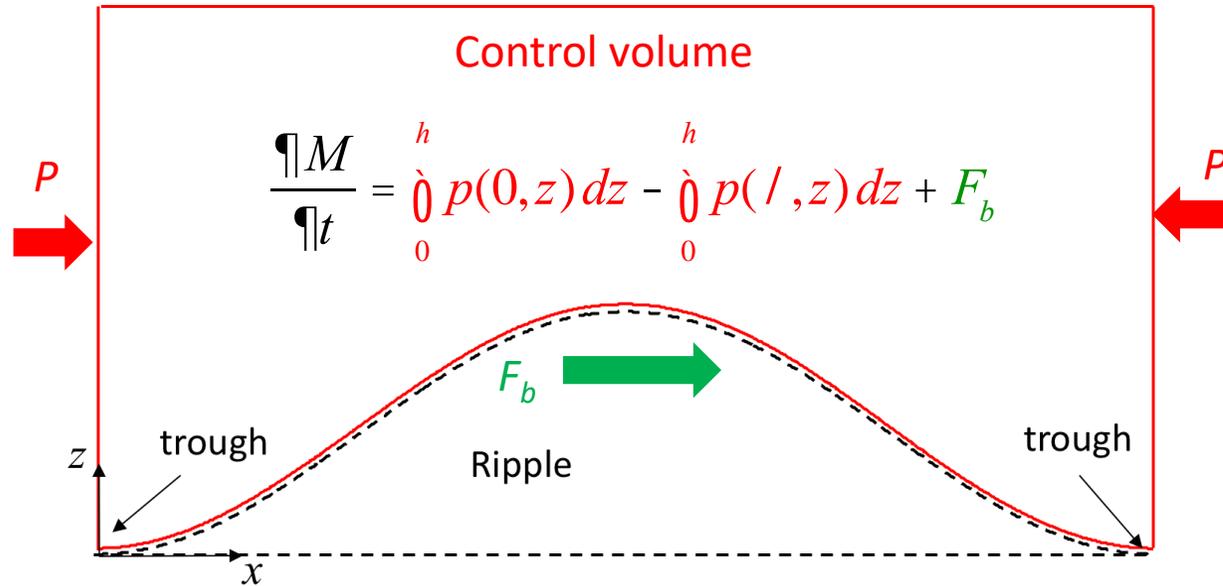
Example: Ta060 ( $U_{bm}=0.60\text{m/s}$ ,  $T=6.25\text{s}$ ,  $H_R=109\text{mm}$ ,  $\lambda=508\text{mm}$ )



**Hypothesis: multiple peaks are associated with coherent vortex motion.**

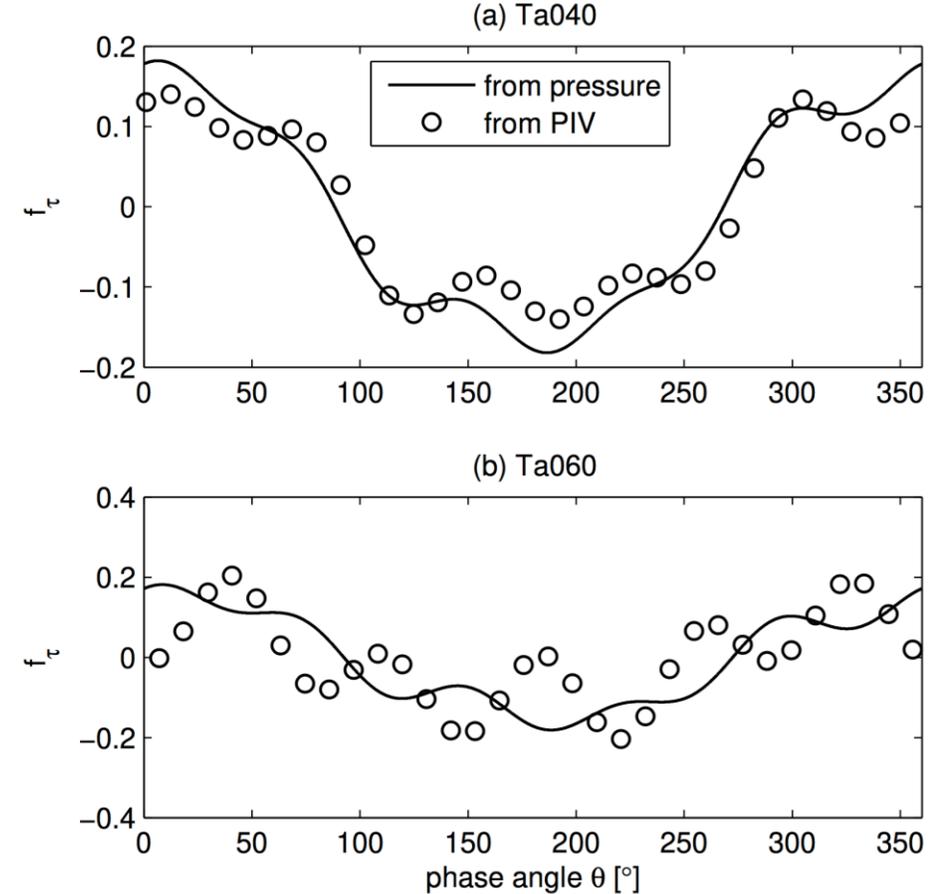


# Comparisons with PIV-based measurements



Some algebra → total bottom shear stress can be estimated with the velocity profile at trough

$$t_b = \frac{\partial}{\partial t} \left[ \int_0^h (u - u_\infty) dz \right] - \frac{\partial u_\infty}{\partial t} \frac{V_{ripple}}{l}$$



Reasonable agreement for major feature (1<sup>st</sup> harmonic), but poor agreement higher-order harmonics.



# Conclusion

- A pressure-based technique is developed for measuring total bottom shear stress over wave-generated vortex ripples.
- Total bottom shear stress is dominated by its first-harmonic Fourier component, which is almost in-phase with the free-stream velocity.
- PIV measurements suggest that coherent vortex motion controls the intra-period variation of total bottom shear stress.
- Good agreement between pressure- and PIV-based measurements for the first harmonic.

