



36TH INTERNATIONAL CONFERENCE ON COASTAL ENGINEERING 2018

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The State of the Art and Science of Coastal Engineering

Field measurements and swash parameterization on beaches

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Outline

- **INTRODUCTION**
- **FIELD DATA**
- **PARAMETERIZATION**
- **RESULTS**
- **CONCLUSIONS AND FUTURE RESEARCH**



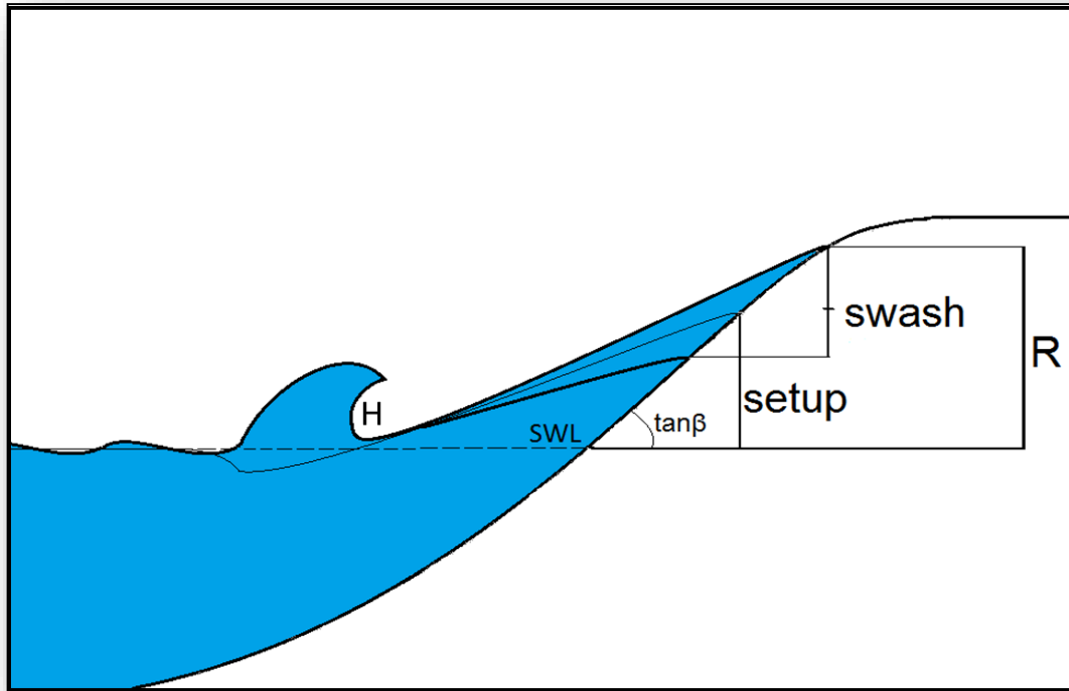
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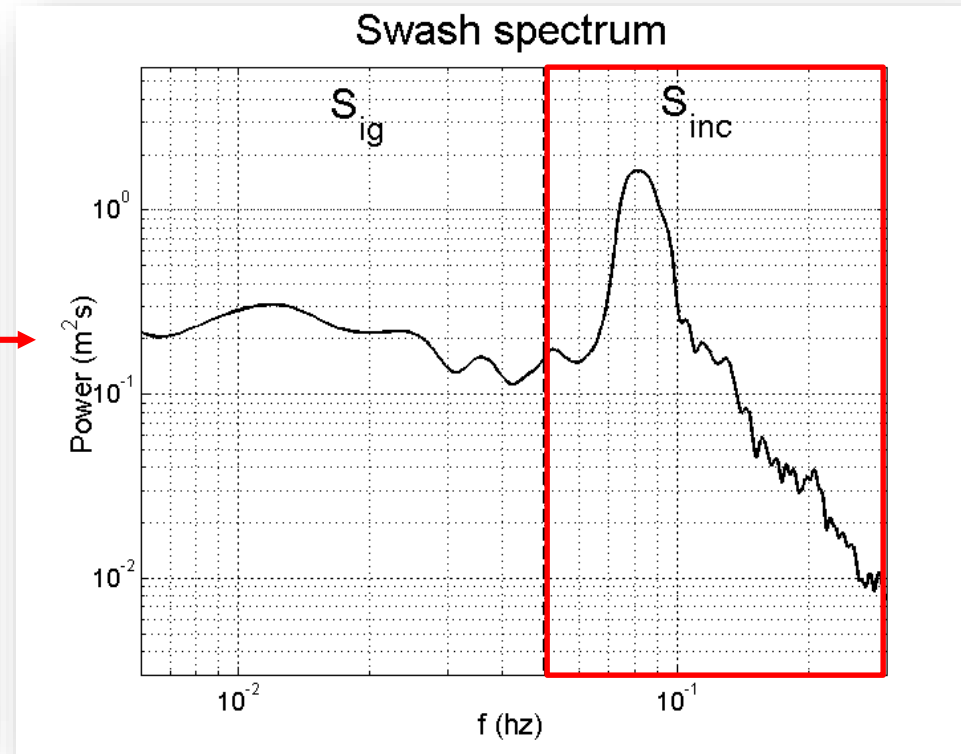
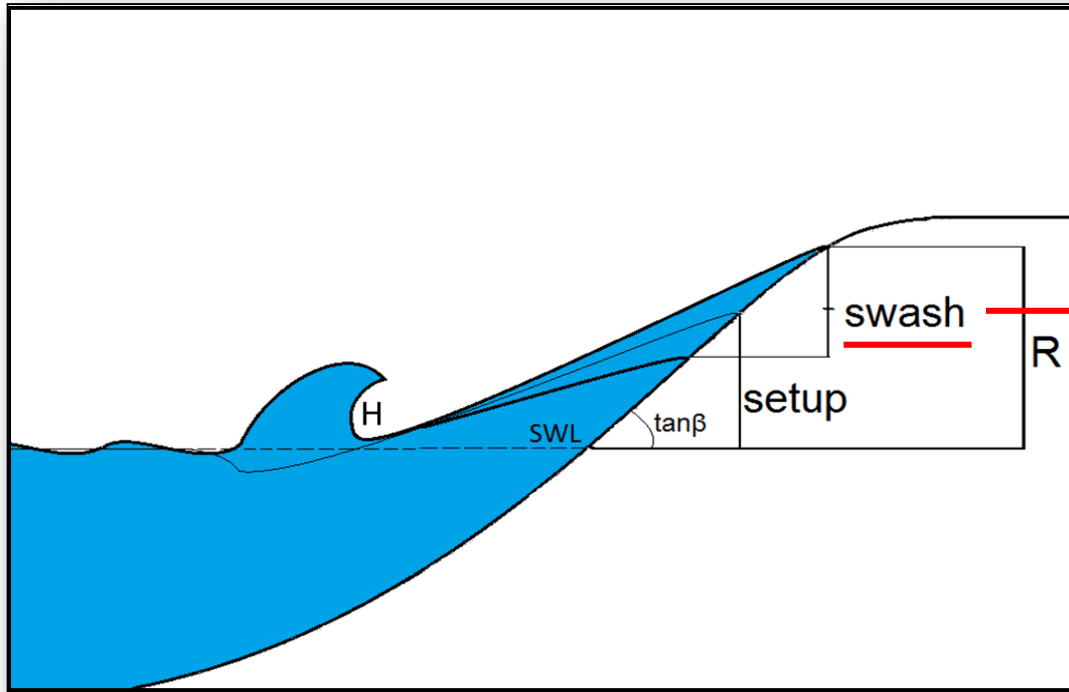
Introduction

- Motivation and basic concepts



Introduction

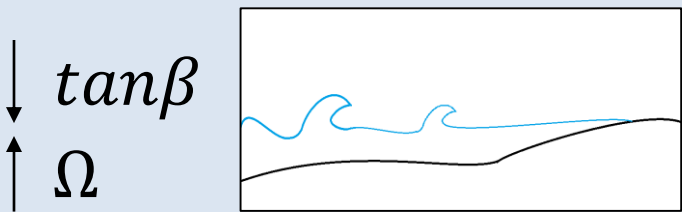
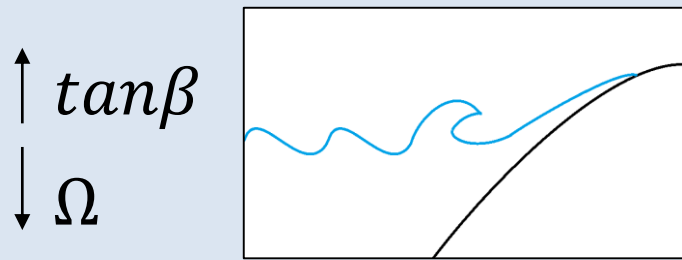
- Basic concepts



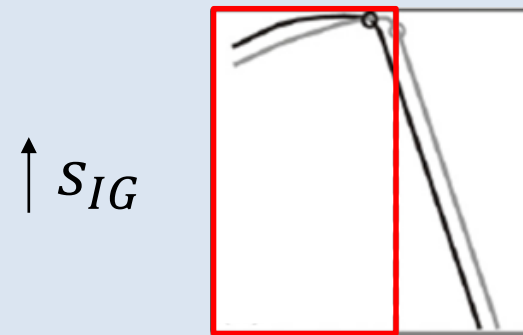
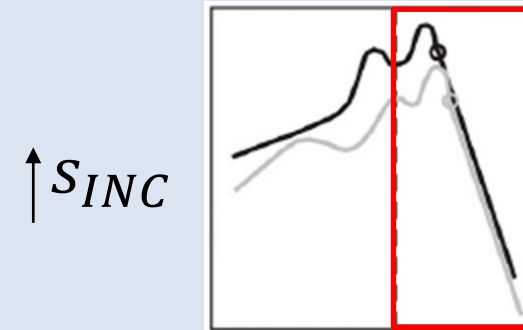
Introduction

- Basic concepts

MORPHODYNAMICS



SWASH



from Hughes et al. (2012)

$$\Omega = H/wT$$



Introduction

- Recent runup formulas: based on parameters established with monochromatic waves
- Hunt's formula

$$\frac{R}{H} = \xi$$

$$\xi = \tan\beta / \sqrt{HL}$$

- Examples

Nielsen and Hanslow (1991)

$$R_{2\%} = 0.188 \tan\beta (H_0 L_0)^{0.5} \quad \text{for} \quad \tan\beta > 0.1$$

$$R_{2\%} = 0.049 (H_0 L_0)^{0.5} \quad \text{for} \quad \tan\beta < 0.1$$



Introduction

- Recent runup formulas: based on parameters established with monochromatic waves
- Hunt's formula

$$\frac{R}{H} = \xi$$

- Examples

Stockdon et al. (2006)

$$S_{\text{setup}} = 0.35 \tan \beta (H_0 L_0)^{0.5}$$

$$S_{\text{inc}} = 0.75 \tan \beta (H_0 L_0)^{0.5}$$

$$S_{\text{ig}} = 0.06 (H_0 L_0)^{0.5}$$



Introduction

- Other studies based on monochromatic waves – based on Miche’s hypothesis
- Carrier and Greenspan (1958), Guza et al. (1984)

$$\frac{S}{H} = \begin{cases} 3 \xi^2 / \pi & \text{for } \xi < \xi_c / 3 & \text{saturation} \\ (2\pi \tan \beta)^{-0.25} \xi & \text{for } \xi_c / 3 < \xi < \xi_c & \text{transition} \\ (\pi / 2 \tan \beta)^{0.5} & \text{for } \xi > \xi_c & \text{reflection} \end{cases} \longrightarrow S = 3 \tan \beta^2 L / \pi$$

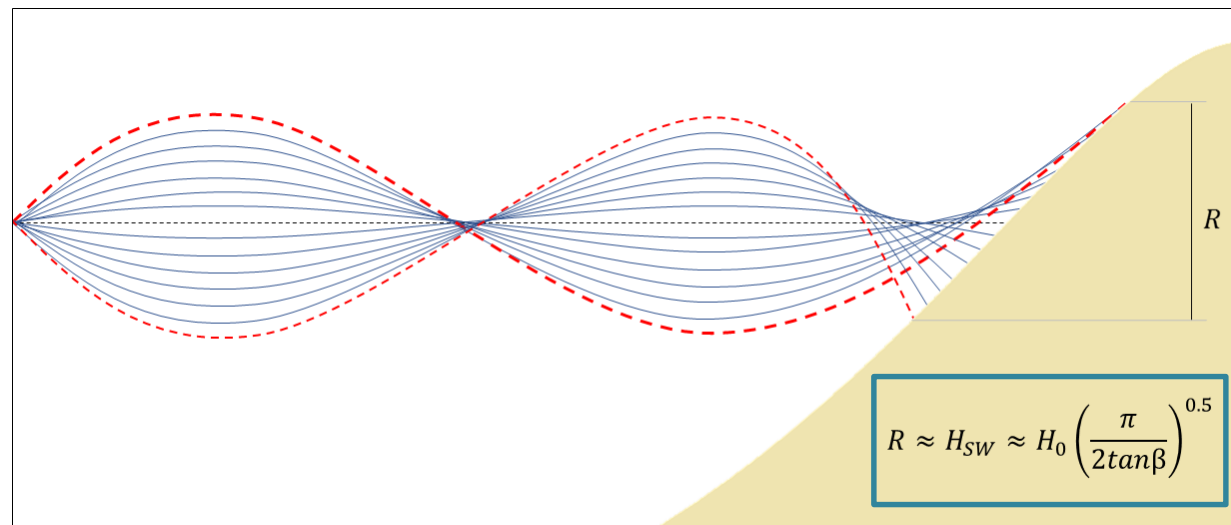
- Application to natural beaches?



Introduction

- 2018: $S_{IG} = f \left[\left(\frac{H_0 L_0}{\tan \beta} \right)^{0.5} \right]$ (Gomes da Silva et al, 2018)
- Role of the beach slope: $\tan \beta^{-0.5}$

$$\frac{S}{H} = \begin{cases} 3\xi^2/\pi & \text{for } \xi < \xi_c/3 & \text{saturation} \\ (2\pi \tan \beta)^{-0.25} \xi & \text{for } \xi_c/3 < \xi < \xi_c & \text{transition} \\ (\pi/2 \tan \beta)^{0.5} & \text{for } \xi > \xi_c & \text{reflection} \end{cases}$$



Introduction

- S_{inc}



Objective: • To assess the **applicability** of the formulas for different conditions of reflection presented by **Guza et al. (1984)** to calculate S_{inc} in **natual beaches**



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Field Data

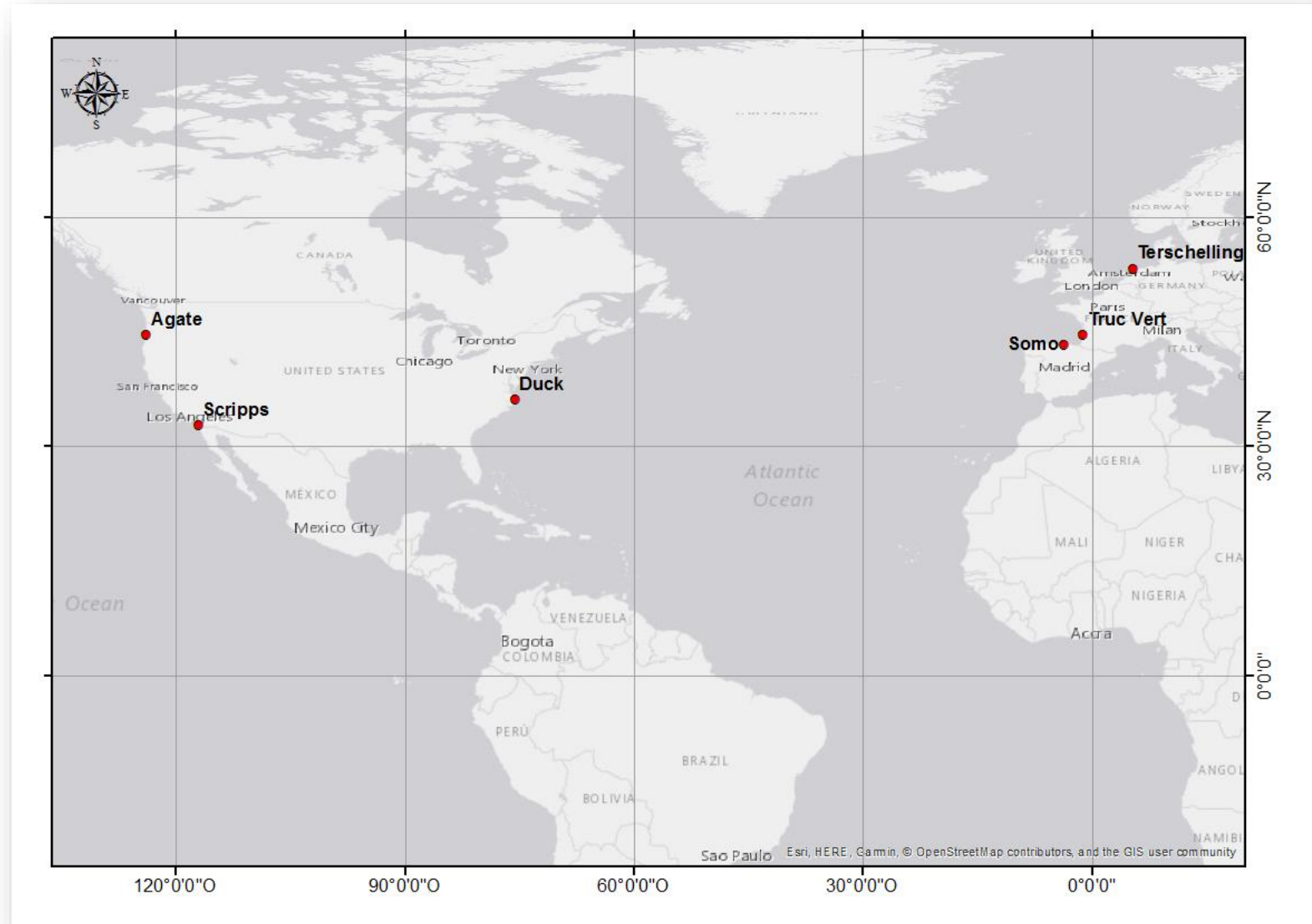
2 sources of data:

i) Compiled dataset

- Stockdon et al. (2006)
US and NL beaches
- Senechal et al. (2011)
Truc Vert Beach

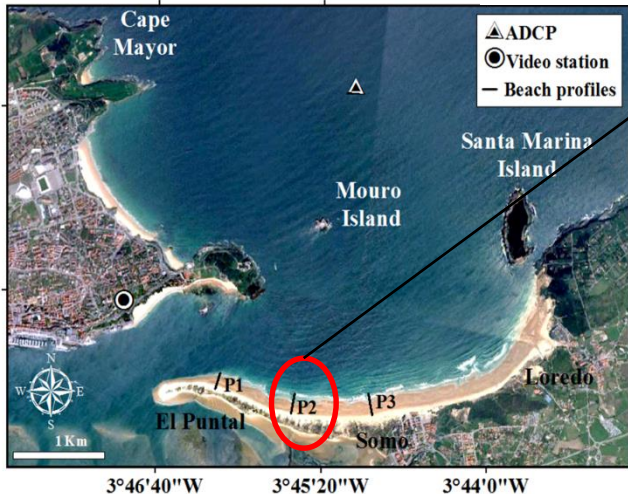
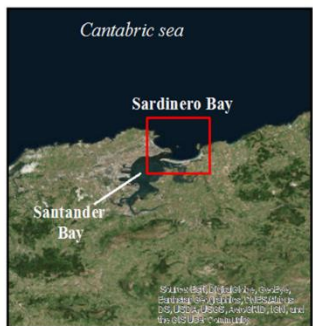
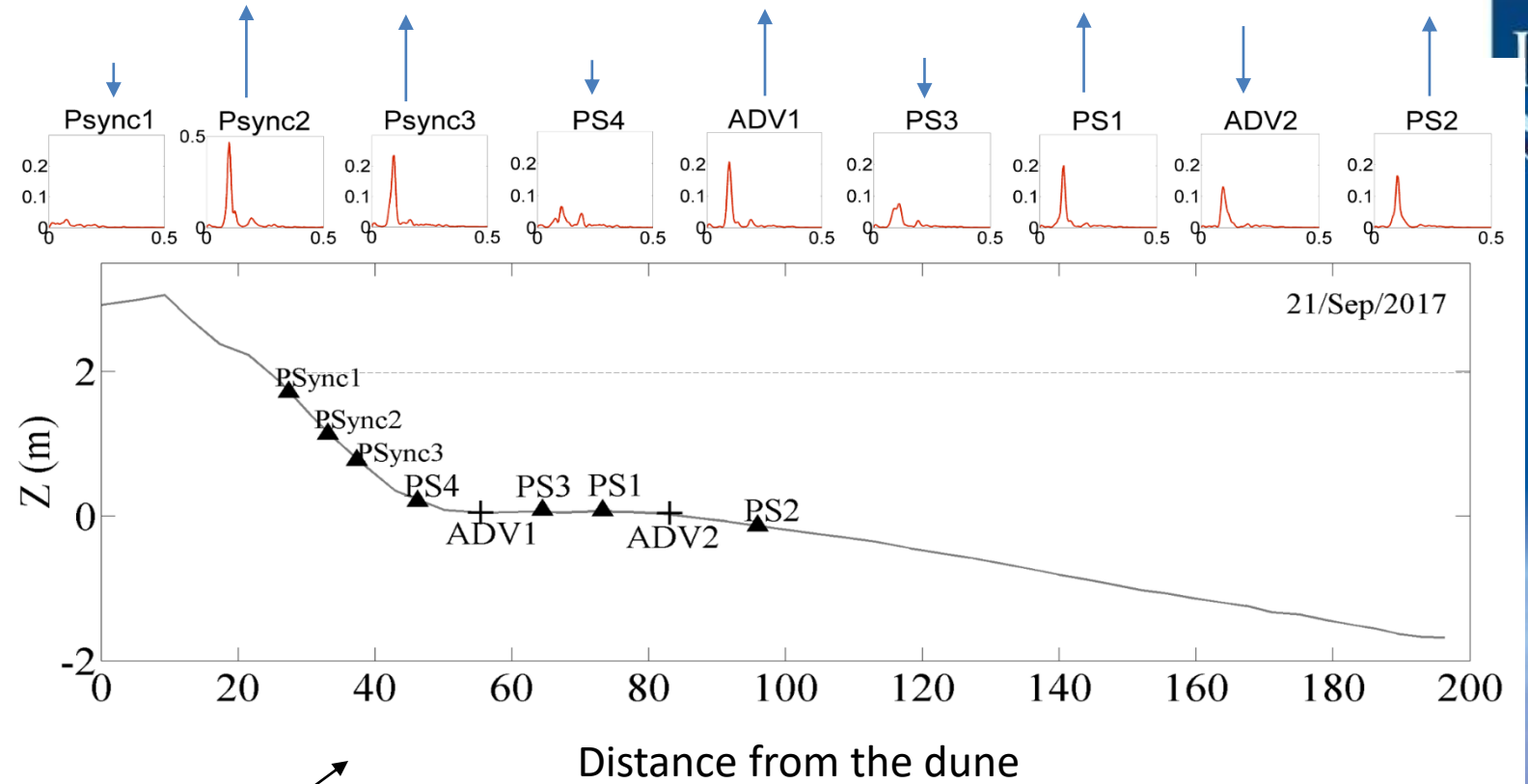
ii) Field Measurements

Somo beach – Noth Spain



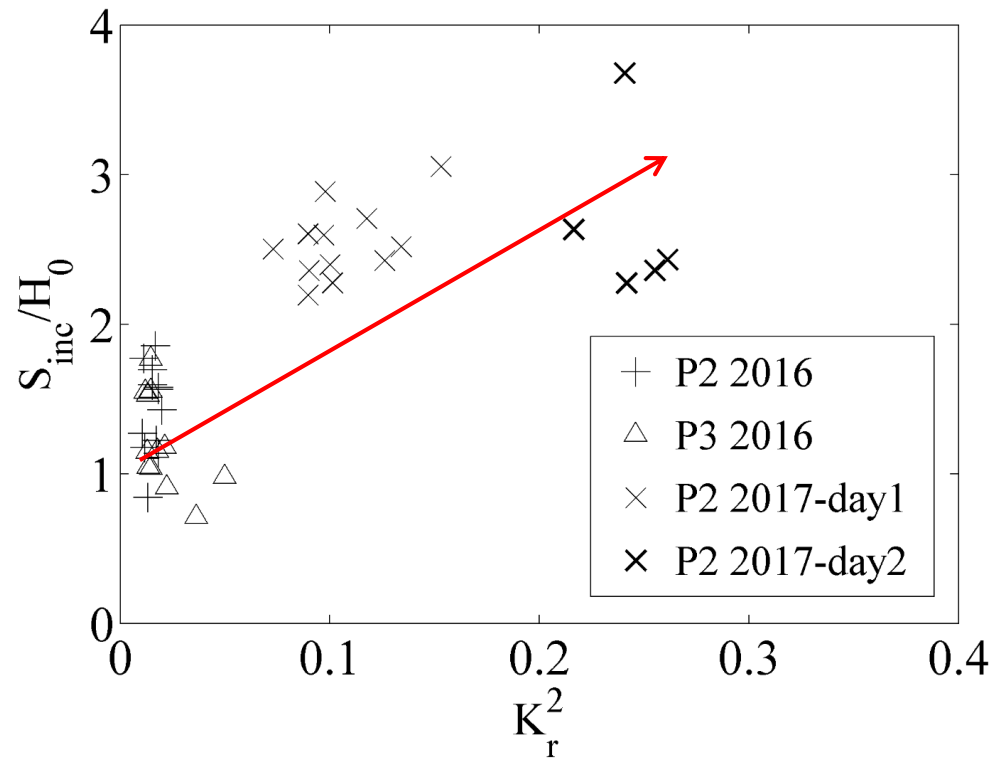
Field Data

Field measurements Somo beach
2016/2017



Field Data

Field measurements Somo beach
MUSCLE-Beach 2017/2016



MORPHODYNAMICS



WAVE REFLECTION



Outline

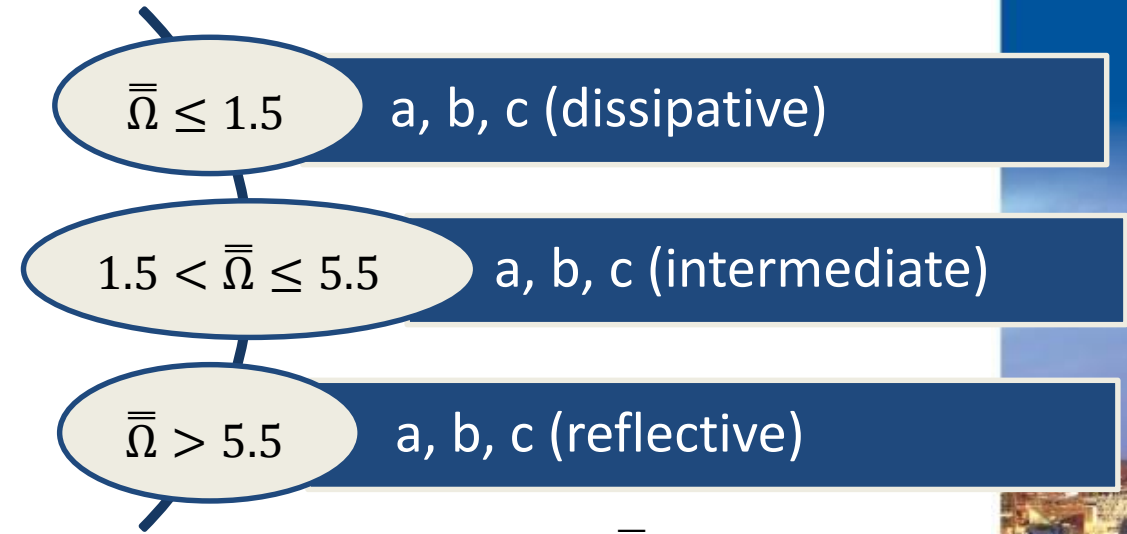
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Parameterization

Basical form:
$$\frac{S_{inc}}{H_0} = a * \tan\beta^b \left(\frac{H_0}{L_0}\right)^c$$

$$\frac{S}{H} = \begin{cases} (3/\pi)\tan\beta^2 \left(\frac{H_0}{L_0}\right)^{-1} & (\text{saturation}) \\ (2\pi)^{-0.25}\tan\beta^{0.75} \left(\frac{H_0}{L_0}\right)^{-0.5} & (\text{transition}) \\ (3/\pi)\tan\beta^{-0.5} \left(\frac{H_0}{L_0}\right)^0 & (\text{reflection}) \end{cases}$$



$\bar{\Omega}$ (Wright et al., 1985)

Hypothesis: *a*, *b* and *c* vary according to the morphodynamic state



Outline

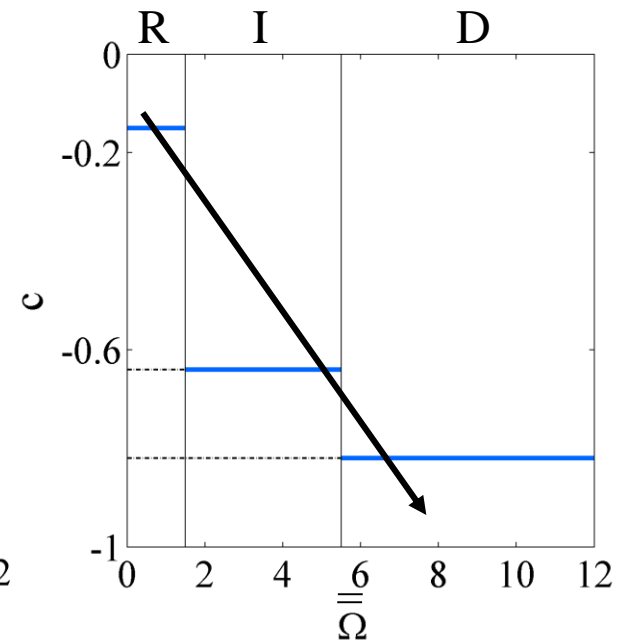
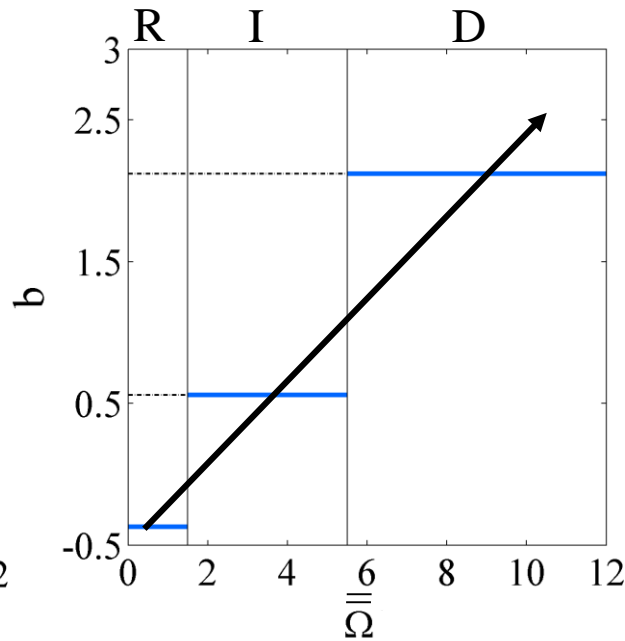
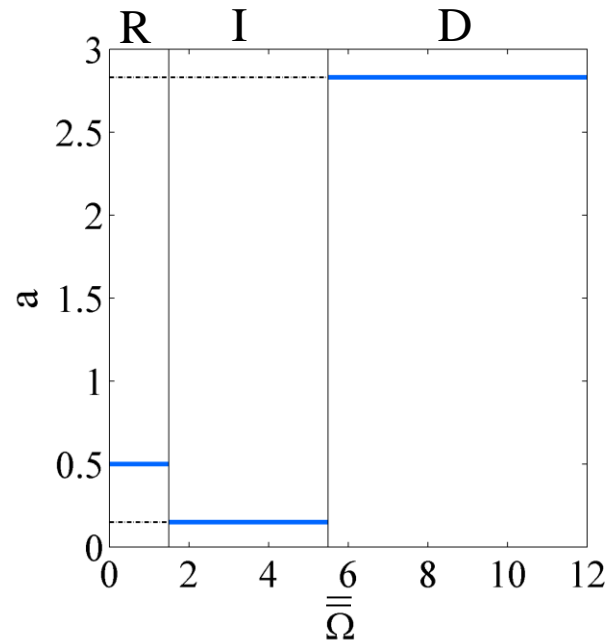
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Results

Coefficients a , b and c

$$\frac{S_{inc}}{H_0} = \mathbf{a} * \tan\beta^{\mathbf{b}} \left(\frac{H_0}{L_0}\right)^{\mathbf{c}} \begin{cases} a = 2.83; b = 2.12; c = -0.82 & (\text{dissipative}) \\ a = 0.15; b = 0.56; c = -0.64 & (\text{intermediate}) \\ a = 0.50; b = -0.37; c = -0.15 & (\text{reflective}) \end{cases}$$



Results

Coefficients a, b and c

$$\frac{S_{inc}}{H_0} = a * \tan\beta^b \left(\frac{H_0}{L_0}\right)^c \begin{cases} a = 2.83; b = 2.12; c = -0.82 & (\text{dissipative}) \\ a = 0.15; b = 0.56; c = -0.64 & (\text{intermediate}) \\ a = 0.50; b = -0.37; c = -0.15 & (\text{reflective}) \end{cases}$$

Guza et al. (1984)

$$b_{\text{reflective}} = 2.0$$

$$c_{\text{reflective}} = -1.0$$

≈

$$b_{\text{saturation}} = -0.5$$

$$c_{\text{saturation}} = 0$$

≈

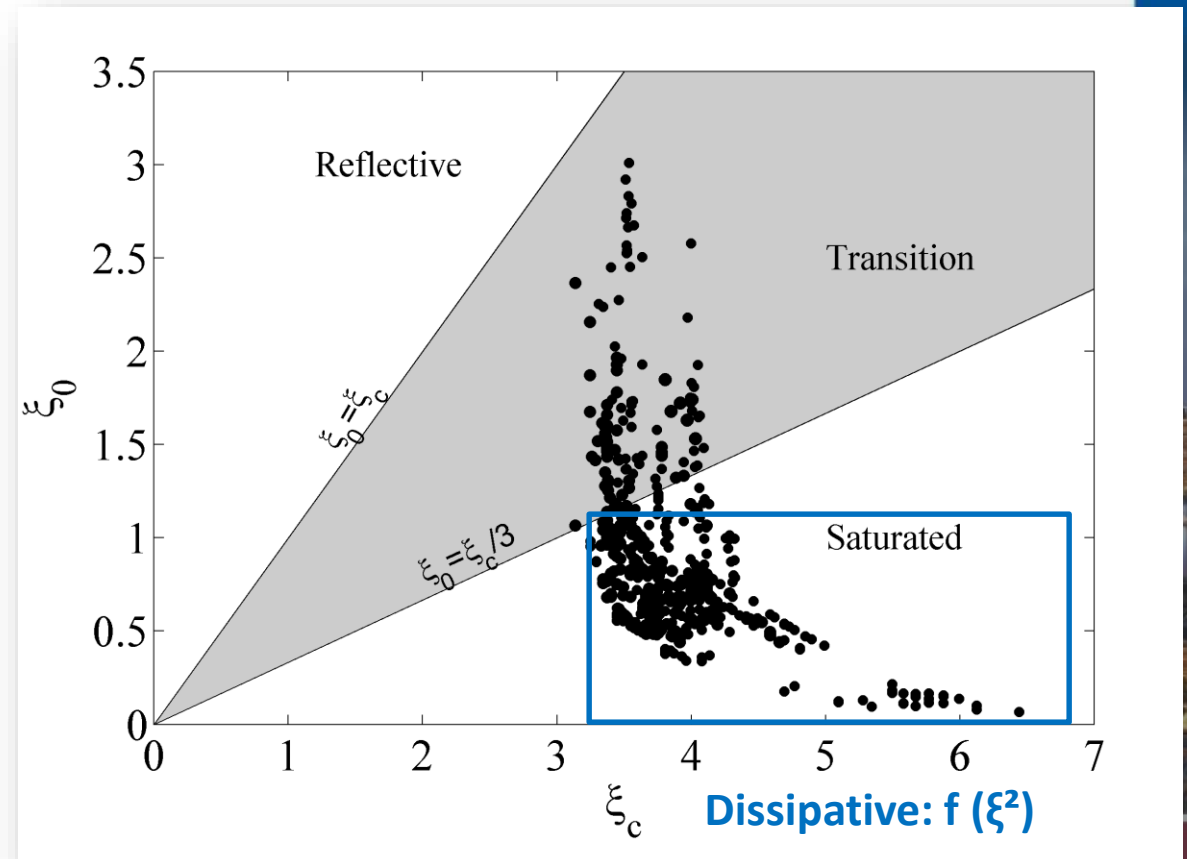
This work

$$b_{\text{reflective}} = 2.12$$

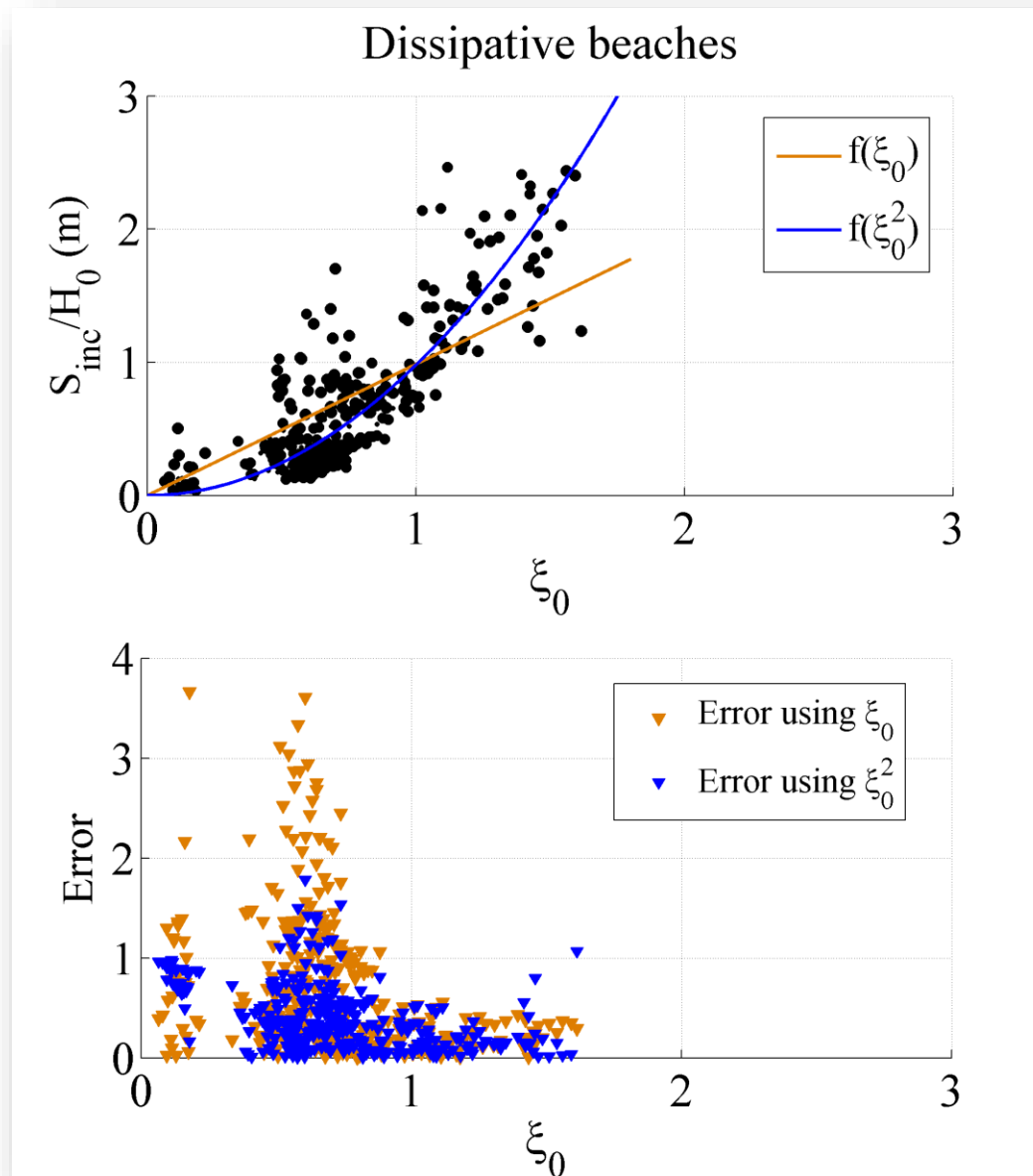
$$c_{\text{reflective}} = -0.82$$

$$b_{\text{dissipative}} = -0.37$$

$$c_{\text{dissipative}} = -0.15$$



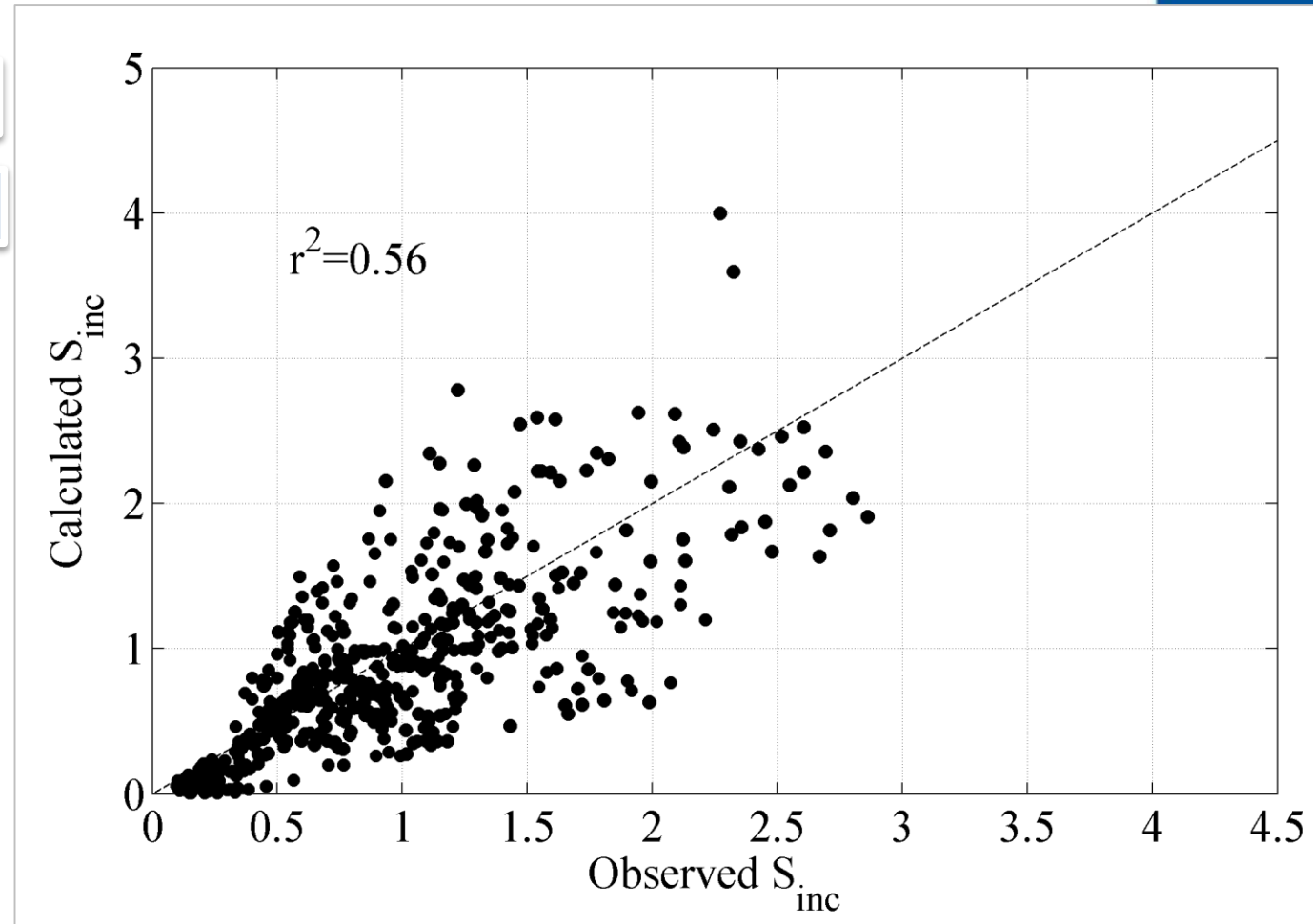
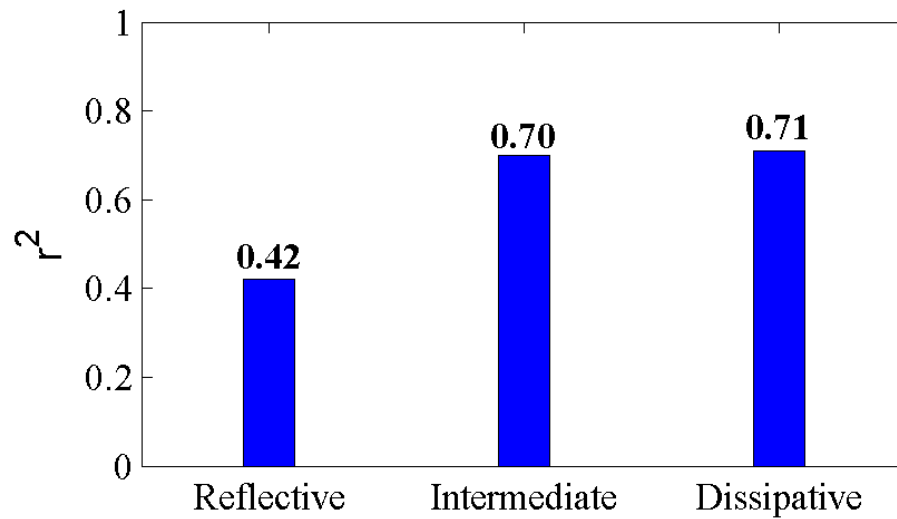
Parameterization



Results

$r^2 = 0.56$ this work

$r^2 = 0.31$ Stockdon et al. (2006)



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Conclusions and future research

- The importance of the amount of reflection on swash values was assessed
- S_{inc} parameterization established assuming a relation between the condition of reflection/saturation and the morphodynamics of the beach using field data
- The role of the foreshore slope and the wave steepness showed to vary according to the morphodynamic state
- Three formulas were obtained to describe the incident swash in reflective, intermediate and dissipative conditions



Conclusions and future research

- Effect of additional parameters (*e.g.* f_s , σ_θ)
- Effect of the tidal level S_{inc}





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Thank you!



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