

Protecting the San Francisco International Airport from Sea Level Rise





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Site Description and Vulnerability





- SFIA has approximately 8 miles of shoreline along San Francisco Bay;
- Protection is needed to prevent flooding;
- Seawalls surround the entire shoreline of SFIA;
- Protection will have to be enhanced to prevent flooding caused by Sea Level Rise.
- The end of the runways are particularly vulnerable due to regulations imposed by the Federal Aviation Administration (FAA) that limits the height of the seawalls.











Sea Level Rise + Subsidence



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SAN FRANCISCO - High emissions (RCP 8.5)

	Probability that sea-level rise will meet or exceed (excludes H++)												
	1 FT.	2 FT.	3 FT.	4 FT.	5 FT.	6 FT.	7 FT.	8 FT.	9 FT.	10 FT.			
2030	0.1%												
2040	3.3%												
2050	31%	0.4%											
2060	65%	3%	0.2%	0.1%									
2070	84%	13%	1.2%	0.2%	0.1%								
2080	93%	34%	5%	0.9%	0.3%	0.1%	0.1%						
2090	96%	55%	14%	3%	0.9%	0.3%	0.2%	0.1%	0.1%				
2100	96%	70%	28%	8%	3%	1%	0.5%	0.3%	0.2%	0.1%			
2150	100%	96%	79%	52%	28%	15%	8%	4%	3%	2%			

SAN FRANCISCO - Low emissions (RCP 2.6)

	Probability that sea-level rise will meet or exceed (excludes H++)											
	1 FT.	2 FT.	3 FT.	4 FT.	5 FT.	6 FT.	7 FT.	8 FT.	9 FT.	10 FT.		
2060	43%	1.4%	0.2%									
2070	62%	4%	0.6%	0.2%	0%							
2080	74%	11%	2%	0.4%	0.2%	0.1%						
2090	80%	20%	3%	1.0%	0.4%	0.2%	0.1%	0.1%				
2100	84%	31%	7%	2%	0.8%	0.4%	0.2%	0.1%	0.1%			
2150	93%	62%	31%	14%	7%	4%	2%	2%	1%	1%		

Source: State of California Sea-Level Rise Guidance 2018 Update by the Ocean Protection Council

1	1515									
	l	Fill Construct	ion	Required Primary S (yea	Time for ettlement ars)	Estimated Settlement due to Fill Placement (inches)				
Runway End	Fill Thickness (feet)	YBM Thickness (feet)	Approx. Construction Year(s)	50%	90%	Total Primary Settlement	Primary Settlement Remaining	Secondary Settlement Remaining		
19	15 - 30	40 - 70	1948, 1961	5 – 15	23 – 55	20 – 50	2 – 5	1 – 2		
28	5 - 20	24 – 50	1963, 1969	3 – 8	10 – 36	19 – 31	2	1 – 2		

Source: Geotechnical Analysis performed by AGS, Inc.



Coastal Engineering Analysis – Water Levels



Analysis

Level, m

Water

- Extracted Tidal Residuals (difference between Measured and Predicted Water Level Values) from Station at Alameda for period between 1979 and 2013;
- Developed Time Series of Astronomical Tide at Oyster Point and San Mateo Bridge using NOAA Established Tidal Constituents for period between 1979 and 2013;
- Added Tidal Residuals onto the Astronomical Tides at Oyster Point and San Mateo Bridge;
- Verified the Methodology;
- Determine Time Series of Water Levels at SFO by Spatially Interpolating the Time Series between Oyster Point and San Mateo Bridge for the period between 1979 and 2013.



Comparison between Measured and Synthetic High Tides at San Mateo Bridge



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Coastal Engineering Analysis – Mesh Generation









Coastal Engineering Analysis – Wave Analysis

Wat

Joint Frequency Distribution (annual) KSFO Period of Observations 1948–2013



Percentage of Occurrence

Total	10.2	2.49	2.77	2.45	2.44	2.18	3.14	2.78	4.5	3.23	3.69	6.26	21.6	21	9.37	1.87	100
30.75																	
22.12													0.01			-	0.04
35.12	0.01								0.05	0.03	0.01	0.02	0.21	0.13	0.01	-	0.48
26.5	0.1	0.02	0.01			0.02	0.05	0.04	0.18	0.13	0.1	0.23	2	1.67	0.56	0.04	5.16
19.87	0.24	0.07	0.06	0.06	0.04	0.1	0.24	0.19	0.43	0.36	0.38	1.05	6.14	6.02	2.66	0.28	18.3
13.25	0.93	0.7	0.76	0.66	0.73	0.82	1.24	0.89	1.27	1.04	1.43	3.06	9.88	9.77	4.42	0.82	38.4
6.625	8.92	1.69	1.94	1.72	1.67	1.24	1.61	1.65	2.56	1.67	1.78	1.9	3.39	3.38	1.73	0.73	37.6
0	N	NNE	NE	ENE	Е	ESE	SE	SSE	S	SSW	SW	WSW	w	WNW	NW	NNW	Total

Wave Analysis Input Data

- Extracted Wind Data from KSFO;
- No Swell Impacts

Wave Modeling Methodology

- Objective: Create a time series of wave parameters for the same duration as the water level data;
- Simulate 1350 steady state runs with combinations of different water levels, wind speeds, and wind directions;
- Linearly interpolate wave parameters for the actual water level, wind speed, and wind direction.

1	13	
er Level (m, NAVD88)	Wind Direction (deg N)	Wind Speed (m/s)
0.00	0.0	0.00
3.28	22.5	17.49
4.10	45.0	23.33
4.92	67.5	29.16
5.74	90.0	34.99
6.56	112.5	40.82
7.38	135.0	46.65
8.20	157.5	52.48
9.02	180.0	58.32
9.84	247.5	-
-	315.0	-
-	337.5	-

360.0



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Coastal Engineering Analysis – Wave Analysis

Example Wave Simulation:

Example Wave Model Results – Significant Wave Height for 30 m/s Wind Speed at 90° and 9.8 ft NAVD88 Water Level



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Coastal Engineering Analysis – Model Verification

Wave Calibration Data

 Calibrated with DHI's Mike21-NSW output hourly time series of wind wave significant wave heights simulated to support FEMA's Floodplain Mapping Program.







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Coastal Engineering Analysis – Total Water Level

Total Water Level Determination

- TWL = Still Water Level + Wave Runup;
- 2% Runup calculated for each hour in the 34-year time series of water level and wave data.
- EVA conducted on Still Water Level + 2% Runup

Extreme Value Analysis for TWL at the end of Runway 28 estimates a total water level of 10.2 feet NAVD88





Sea Level Rise Remedial Solutions

Flooding at Runway End

- Ponding from Wave Overtopping
- Expected to Increase with SLR
- Cannot raise elevations of the seawall due to FAA restrictions

Current Elevations at End of Runway 28





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Short- term SLR Proposed Solution

- Still water level not expected to reach top of embankment;
- Major concern is increase wave overtopping;
- Grade the existing area to allow waves to runup and drain back to Bay.

Sea Level Rise Remedial Solutions

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Long-Term Sea Level Rise Solution

- Must minimize 'footprint' within San Francisco Bay waters;
- Propose to enclose runway ends with a sheet pile breakwater offset a distance sufficient enough to accommodate the required seawall elevation.







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THANK YOU!

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