Chapter 17

WAVE ENERGY AND LITTORAL TRANSPORT

José Castanho

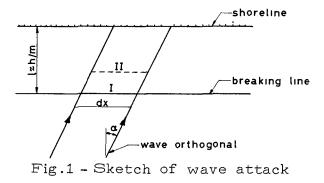
Engineer, Beaches and Harbors Division Laboratório Nacional de Engenharia Civil, Lisboa, Portugal

1. GENERAL

As is known the breaking of oblique waves generates currents roughly parallel to the shore line usually designated by long shore currents. The intensity of these currents which are present ent almost exclusively between the breaking line and the shore de pends on the characteristics of the waves (angle of approach, height and period) and on the characteristics of the shore(slope and roughness).

A certain amount of energy \underline{E} is transmitted by the break ing wave along its direction of propagation. As this is a transmitted energy, it is possible to speak about its component paral lel to the shoreline which would be indicated by \underline{E} sen α , being the angle of approach of waves, i.e. the angle that crests make with the shoreline.

Let us consider the breaking line and two near orthogonals distant dx from one another (fig.1).



A fraction of the energy flowing through the breaking line remains in the longshore current from which it will flow out in a continuous or concentrated form (rip currents); another part is dissipated by friction in the bottom whilst the remaining is lost by breaking (turbulence).

COASTAL ENGINEERING

The distribution of the difference between the energy flow ing in at section I and flowing out at section II by these thre fractions depends firstly on the type of breaking(spilling or plun ing) and secondly on the characteristics of the wave and of th shore.

That distribution varies also along the zone where the lon shore current is present, i.e. between the breaking line and th shore.

2. DISTRIBUTION OF TRANSMITTED ENERGY

2.1 Energy dissipated in friction losses - Let us assum that the average velocity \underline{V} of the longshore current betwee the breaking line and the shore^{*} is known. Denoting by \underline{k} th friction factor and assuming that the friction force is propor tional to the square of the velocity, the energy dissipated in beach length dx will be proportional to the third power of th velocity and can be written

$$E_{d} = k \rho V^{3} dx$$
or
$$E_{d} = k \rho V^{3} \frac{h}{m} dx$$
(1)

where ρ represents the unit mass of water, <u>m</u> being the slope c the beach and <u>h</u> the breaking depth.

Assimilating the breaking waves to solitary waves arriving each T seconds (T being the period)^{XX} the transmitted er ergy parallel to the shore, per second is

$$E_{t} = \frac{2, 2g H^{3}}{T} \operatorname{sen} \alpha \cos \alpha \, dx \qquad (2)$$

H being the wave height.

According to some authors it is preferable to consider r riodic waves of which the deep-water characteristics are knowr

260

⁽x) For the present purpose it is sufficient to consider an ave age value of the velocity of the current. In a more detaile study now under way the variation of the velocity of the long shore current in the surf zone is taken into account.

⁽xx)Munk:"The solitary wave theory and its application to su problems". Annals of the New York Academy of Sciences Vol.51 - 1949

Anyhow if these characteristics were known, it would always be possible to calculate the solitary wave which each T seconds transmits the same amount of energy than the periodic wave in deep water.

Therefore we shall continue to assimilate breaking waves to solitary waves.

The ratio <u>s</u> of the energy dissipated in friction losses in the bottom (eq.1) and the component of the transmitted energy parallel to the shore (eq.2) is

$$s = \frac{k \rho V^{3} h/m dx}{2,2 gH^{3} sen \alpha \cos \alpha dx/T}$$
(3)

Writing the velocity of propagation of the breacking solitary wave $C = \sqrt{g(h + H)}$ with H = 0,78 h and considering the steep ness of the wave $\delta = \frac{H}{L} = \frac{H}{CT}$, equation (3)

becomes

$$s = \frac{(V/C)^3}{0.38 \operatorname{Asen} 2\alpha}$$
(4)

where <u>A</u> is the dimensionless parameter $A = \frac{m\delta}{k}$

2.2 Energy contained in the longshore current - According to the theory of the solitary wave, the volume of water \underline{Q} leaving the breaking wave in each strip of width dx is

$$Q = 2 h^{2} \cos \alpha \, dx \tag{5}$$

 $\underline{\vee}$ being the mean velocity of this volume of water, its kinetic energy will be

$$E_{c} = \frac{1}{2} Q V^{2}$$
 (6)

The ratio \underline{t} of the kinetic energy of the longshore current (eq.6) to the component of the transmitted energy parallel to the shore can be written, taking equation(5) into account,

$$t = \frac{(V/C)^2}{0.64 \, \text{sen}\,\alpha} \tag{7}$$

2.3 Energy dissipated in wave breaking(turbulence) - Th energy fraction dissipated in breaking is

$$r = 100\% - (s + t)\%$$

3. CALCULATION OF THE MEAN VELOCITY OF THE LONGSHORE CURRENT

Let us apply the momentum method for calculating the mea velocity of the longshore current.

Being Q the volume of water carried by the solitary breating wave and C_b the velocity of propagation, the component pathel allel to the shore of the transmitted rate of momentum will be

$$Q_{h} \times C_{h} \quad sen \alpha \cos \alpha \, dx/T$$

 \underline{V} being the mean velocity of the longshore current, the value $Q_{\rm b}$ keeps a momentum towards the shore equal to

$$Q_{b} V \cos \alpha dx/T$$

According to the momentum theorem, the variation of t. rate of momentum is equal to the friction force. Therefore will be

$$\frac{Q_{b}C_{b}}{T} = \frac{Q_{b}V\cos\alpha}{T} - k\rho V^{2} \cdot \frac{h}{m} \qquad ($$

Computing \underline{V} from equation (8) and substituting its value equations (4) and (7), it is possible to determine the values of and \underline{t} , i.e. the fractions relative to the ingoing energy respectively of the energy dissipated in friction losses in the bottom a of the energy contained in the longshore current.

Nevertheless if equation (8) was applied as written above we should find that for some values of s+t>100% which is possible.

This lead us to some considerations on the momentum ac ally available to generate the longshore current.

In the present paper only the case of beaches with a vegentle slope (say 2 per cent or less) will be considered, it bei

WAVE ENERGY AND LITTORAL TRANSPORT

assumed that wave breaking is gradual so that the wave resumes its shape at each moment, remaining practically symmetric. This amounts to assuming that the wave height decreases linearly with depth and that the decrease of transmitted energy corresponds exactly to the energy progressively lost in wave breaking. The wave height thus follows constantly the law H = 0.78 h.

In a study now in progress we have approached the case of a breaking wave giving rise to a bore.

Let us consider then two neighbouring sections I and II (fig.1). The change of momentum between them will be

$$dM = d(QC) = Q dC + c d Q$$

From $Q = 2 h^2$ and $C = 1.78 g h \frac{1}{2}$ (solitary wave), the expressions dQ = 4 h dh and $dC = \frac{1}{2} Ch^{-1}dh$ are obtained. Hence

$$\int_{h_{1}}^{h_{2}} C dQ = \frac{4}{5} (Q_{1}C_{1} - Q_{2}C_{2})$$

$$\int_{h_{1}}^{h_{2}} Q dC = \frac{1}{5} (Q_{1}C_{1} - Q_{2}C_{2})$$

This lead us to suggest that between sections I and II the momentum available to generate the longshore current is only the volume d Q of water available in the section under consideration times the velocity of propagation C, the momentum available between the breaking line and the shore being

$$\int_{h_b}^{0} C dQ = \frac{4}{5} Q_b C_b$$

The amount $\int_{hb}^{0} QdC = \frac{1}{5} Q_b C_b$ would correspond to a lost momentum since the wave flowing out at section II has a velocity of propagation C+dC instead of velocity C which corresponds to the wave flowing in at section I.

COASTAL ENGINEERING

The momentum theorem will be written now

$$\frac{4}{5} \frac{Q_b C_b \operatorname{sen} \alpha \cos \alpha}{T} - \frac{Q_b V \cos \alpha}{T} = k \rho V^2 \frac{h}{m}$$
(9)

Solving equation (9) with respect to V and taking into ac count that $Q_b = 2 h^2$, $C_b = 1.78g h^{\frac{1}{2}}$ and $\delta = \frac{H}{C_b T}$, the followin dimensionless equation results

$$\frac{V}{C_{b}} = 1.30 \cos \alpha A \left[\sqrt{1 + \frac{1.22 \tan \alpha}{A}} - 1 \right]$$
(10)

where $A = \frac{m\delta}{k}$

Thus for a given angle of approach , the relative veloc ity $\frac{V}{C_b}$ is a function of the parameter A = $\frac{m\delta}{k}$ alone.

4. CALCULATION OF THE ENERGY FRACTIONS DISSI PATED IN FRICTION LOSSES AND BREAKING LOSSE

By substituting the value of $\frac{V}{C_b}$ given by equation (10), is possible to obtain <u>s</u> and <u>t</u> from equations (4) and (7).

Figures 2 and 3 show the values of <u>s</u> and <u>t</u> for angles $10^{\circ}, 30^{\circ}, 45^{\circ}, 60^{\circ}$ and 80° in a function of the parameter $A = \frac{m}{1}$

Figure 2 shows that the fraction <u>s</u> of the energy dissipted in friction losses in the bottom reaches a maximum for a cetain value of <u>A</u> which varies in accordance with .

On the other hand, for values of <u>A</u> not exceeding 0.3, the fraction <u>s</u> is a function of the angle α , reaching a maximum new 45° to 60°.

Figure 3 shows that energy dissipated in breaking losses always important (between 85 and 95%) for angles $\alpha = 10^{\circ}$. Fo greater angles the fraction <u>r</u> is important for small values of A decreasing regularly as <u>A</u> increases.

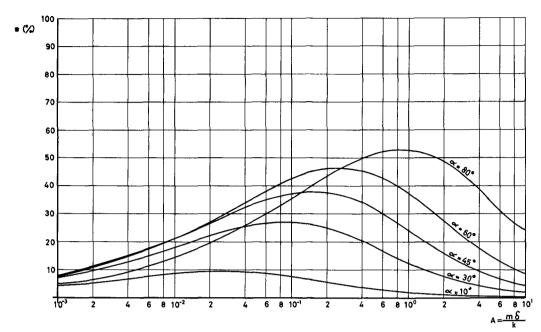
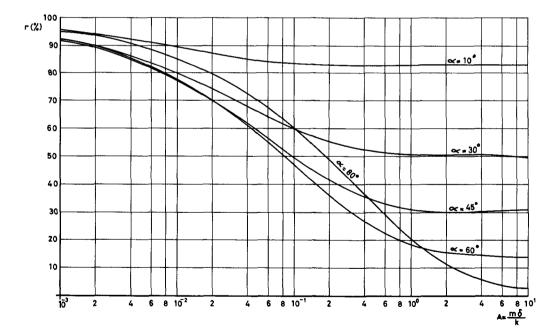


FIG 2 -- PERCENTAGE OF ENERGY DISSIPATED BY FRICTION





266 WAVE ENERGY AND LITTORAL TRANSPORT

5. SOLID DISCHARGE

For constant values of \underline{m} and \underline{k} the shape of the curves $s = f(\frac{m\delta}{k})$ or $s = f(\delta)$ is very similar to that of curve $Q_{solid} = f(\delta)$. The same applies to $s = f(\alpha)$, a maximum being reacled between $\alpha = 45^{\circ}$ and $\alpha = 60^{\circ}$.

This seems to suggest that the solid discharge can be di mensionlessly expressed as a function of \underline{s} . In a first attempt Q_{solid} could be considered as proportional to \underline{s} .